

DOCKET ITEM #13 Resource Protection Area Exception Request 4898 West Braddock Road - Newport Village

Application Public Hearing and consideration of a request for an exception, pursuant to Zoning Ordinance Section 13-119, to allow a new encroachment of up to 4,791 square feet of impervious surface in a Resource Protection Area (RPA) in connection with the future redevelopment of a multifamily residential property	General DataPlanningCommissionHearing:City CouncilHearing:Image: Commission	February 4, 2020 N/A
Address: 4898 West Braddock Road	Zone:	RA/Multifamily
Applicant: UDR, represented by Kenneth W. Wire, attorney	Small Area Plan:	Alexandria West

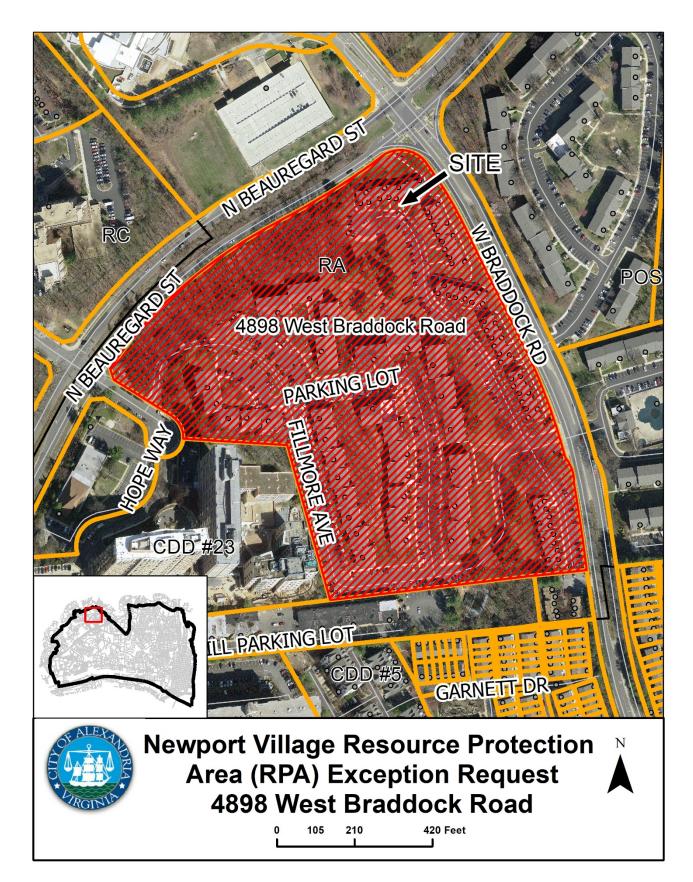
Staff Recommendation: DENIAL

Staff Reviewers:

Jesse Maines, Stormwater Management Division Chief, T&ES, <u>jesse.maines@alexandria.va.gov</u> Melanie Mason, Principal Planner, T&ES, <u>melanie.mason@alexandriava.gov</u> Maya Contreras, Principal Planner, P&Z, <u>maya.contreras@alexandriava.gov</u>

Environmental Policy Recommendation:

At the December 16, 2019, Environmental Policy Commission (EPC) meeting, the members of the Commission voted 6-5 to support the staff recommendation to deny the exception request at 4898 W. Braddock Road.



I. <u>SUMMARY</u>

A. Recommendation

Staff recommends **DENIAL** of the UDR (applicant) request for an exception to the requirements of Article XIII of the Alexandria Zoning Ordinance for a new encroachment of 4,791 square feet of new impervious surface in a Resource Protection Area (RPA) associated with DSUP2018-0018 for the Newport Village redevelopment. The proposed layout provided in the request for exception does not satisfy requisite criteria in the Zoning Ordinance to grant the exception as discussed below.

B. General Description

The applicant has proposed to redevelop a portion of its existing multifamily residential community, known as Newport Village, with two new multifamily buildings and an underground garage on a portion of the property addressed as 4898 W. Braddock Road (the "site") near the corner of North Beauregard Street and West Braddock Road. The redevelopment plans are in the early stages of City staff's review and have not progressed to public hearings before the Planning Commission or City Council. While the draft Concept 2 plan showing the proposed encroachment has not been formally submitted to the City for review, select sheets were submitted to the City for consideration of the Water Quality Impact Assessment (WQIA), as required for the request for an exception.

A natural, perennial stream is located within the median of North Beauregard Street near the future project site. On each side of this stream, encompassing Beauregard Street and portions of adjacent properties, is a previously identified 100-foot RPA buffer. The RPA buffer is depicted on the City's 2004 RPA map consistent with the Chesapeake Bay Act and attendant regulations as incorporated into Article XIII of the Zoning Ordinance, given that the City is located within the Chesapeake Bay watershed. The RPA extends approximately 39-59 feet into the subject property.

The applicant is requesting an exception to the following section of the Zoning Ordinance:

• 13-107 - Development, redevelopment, and uses permitted in RPAs.

II. <u>BACKGROUND</u>

A. Procedural Background

The City has been delegated as the authority by the Commonwealth of Virginia to safeguard natural areas through the application of the Chesapeake Bay Preservation Act ("Bay Act") and attendant regulations through incorporation of these requirements into Article XIII of the Alexandria Zoning Ordinance, the Environmental Management Ordinance (EMO).

RPA Definition

RPAs are required under Virginia Administrative Code Chapter 9VAC25-830: Chesapeake Bay Preservation Area Designation and Management Regulations and the Bay Act. The overarching goal of the Bay Act is to protect RPAs to the maximum extent practicable and prevent substantial damage to the water quality of the Chesapeake Bay and its tributaries.

Per Section 13-105, RPAs consist of sensitive land that has either an intrinsic water quality value due to the ecological and biological processes such land performs or that is sensitive to uses or activities such that the use results in significant degradation to the quality of state waters. In their natural condition, these lands provide for the removal, reduction, or assimilation of nonpoint source pollution entering the bay and its tributaries. An area of land that includes any one of the following land types shall be considered to be within the RPA:

- (1) Tidal wetlands;
- (2) Tidal shores;
- (3) Nontidal wetlands connected by surface flow and contiguous to tidal wetlands or water bodies with perennial flow;
- (4) Water bodies with perennial flow.
- (5) A buffer area of 100 feet (measured from top of bank) located adjacent to and landward of the components listed above and along both sides of any water body with perennial flow, to include the environmental feature, constitutes the RPA.

In this case, the RPA feature is attributed to the perennial stream located in the median of North Beauregard and its associated 100-foot buffer.

Allowable Redevelopment within the RPA

Per Section 13-107(C)(2), redevelopment may be allowed provided that the following criteria are met:

- (1) There is no increase in impervious surface cover;
- (2) There is no further encroachment within the RPA; and
- (3) The proposed redevelopment is consistent with the city master plan.

Certain activities such as trails, utility work and roads are permitted in the RPA provided any impervious area is restricted to the minimum amount necessary for the activity.

Exception Request Review Process

The Zoning Ordinance allows for the filing of requests with the City for exceptions to RPA provisions. Consistent with Section 13-119 (B), the Department of Transportation & Environmental Services (T&ES) reviews such exception requests based on criteria included therein and renders a recommendation. Per Section 13-119(G), requests for an exception to the RPA provisions of the EMO are first heard by the Environmental Policy Commission (EPC). The EPC can recommend support, denial or modification of the exception. The EPC recommendation, along with T&ES staff recommendation, are then brought to the Planning Commission, which is the body that will make the formal determination on the request.

Exception Criteria to be Considered

To grant an exception to the RPA provisions, Section $13-119(B)^1$ of the EMO, the reviewing body must find that the applicant has proven each of the following criteria by a preponderance of the evidence:

- (1) Granting the exception will not confer upon the applicant any special privileges that are denied to other property owners in the CBPA [Chesapeake Bay Preservation Area] overlay district;
- (2) The exception is not based upon conditions or circumstances that are self-created or selfimposed, nor does the exception arise from conditions or circumstances either permitted or noncomplying that are related to adjacent parcels;
- (3) The exception is the minimum necessary to afford relief;
- (4) The exception will be consistent with the purpose and intent of the overlay district, and not injurious to water quality, the neighborhood or otherwise detrimental to the public welfare;
- (5) Reasonable and appropriate conditions are imposed, as warranted, to prevent the allowed activity from causing degradation of water quality.

Economic hardship alone is not sufficient reason to grant an exception per Section 13-119(C). The above criteria are the only ones that may be considered when reviewing the merits of an exception request.

B. Site Context

The site of the future new redevelopment consists of 4.19 acres and is located southeast of the intersection of North Beauregard Street and Braddock Road. A natural, perennial stream is located nearby within the median of North Beauregard Street with portions being previously piped. This portion of the stream channel averages 9 feet wide, is approximately 850 total linear feet, and is a tributary of Four Mile Run. On the north and south banks, the stream is bound by North and South Beauregard Street. On each side of this stream there is a 100' RPA buffer measured from the top of bank spanning Beauregard Street and extending into the adjacent properties, with the RPA extending approximately 39-59 feet into the Newport Village redevelopment site. As it exists today, that site consists of garden style apartments, an access road and parking lot, sidewalks, and apartment amenities. The area of total RPA on the site is approximately 30,492 square feet and currently consists of 432 square feet of impervious area, 9,583 square feet of pervious area, and 20,473 square feet of forested area.

C. Exception Description

Per the EMO, redevelopment may be only allowed in the RPA if there is no increase in impervious surface cover, there is no further encroachment within the RPA, and the proposed redevelopment is consistent with the city master plan.

¹ The provisions of the EMO are based upon the regulations found in 9VAC25-830-150 of the Virginia Administrative Code.

The applicant has requested an exception to Section 13-107 of the Alexandria Zoning Ordinance to allow a new encroachment in the RPA and an increase in impervious surface cover.

The proposed new encroachment would consist of 4,791 square feet of new impervious surface for a total encroachment (including existing impervious surface) in the RPA of 5,226 square feet of impervious surface. The existing building that is partially located in the RPA was constructed in 1968, prior to the existing RPA protection requirements for perennial streams enacted in 2004. The total proposed disturbance in the RPA in connection with the redevelopment, including earthwork, utility work, tree removal, and the previously mentioned new and existing impervious surface area (consisting of buildings and sidewalks) would be approximately 15,681 square feet.

III. STAFF ANALYSIS

A. Compliance with Environmental Management Ordinance

The requested exception does not meet the criteria outlined in Section 13-119 of the EMO, and therefore does not meet the requirements for approval.

The request for an additional 4,791 square feet of new impervious surface in an RPA is considered an extremely large exception request. In general, exceptions are intended to be used only for very small areas where no other option for development is available.

The applicant has also stated that the encroachment is necessary for development because it is not economically feasible to develop the site outside of the RPA. However, economic hardship alone cannot be the only reason for an exception to be approved. No other reason has been given as to why the encroachment is necessary for development.

Exception Criteria (1). The exception must be the minimum necessary to afford relief.

Projects proposing encroachments into the RPA must demonstrate that the project is only considering the minimum encroachment necessary to afford relief. According to state guidance on exceptions, when considering the minimum necessary to afford relief, the size of the structure, the types of proposed structures, and the placement of the structures in relation to the size, layout and location of the lot or parcel must be considered. If alternative location, sizing, or orientation options to avoid the need for an exception are available, then the finding of "minimum necessary to afford relief" has not been met.

The existing site is already developed with a much smaller encroachment in the RPA constructed prior to Bay Act requirements which demonstrates that the site has the ability to be developed with a smaller footprint in the RPA. In addition, multiple iterations of different layouts for the redevelopment should have been provided for consideration to demonstrate that this draft Concept 2 configuration included in the request for exception is the minimum amount of impervious surface needed for development. No such layouts have been provided as part of the official exception request; however, in previous concept submissions, the applicant provided

layouts that removed the existing building from the RPA and added no new impervious area to the RPA. The existence of these layouts in previous submissions clearly demonstrates it is possible to develop the site without additional encroachments.

In addition, in a letter dated September 5, 2019 ("RPA Waiver Request") to Karl Moritz and Yon Lambert, Kenneth Wire, the attorney for UDR stated: "If the EPC finds that is cannot recommend support of the proposed encroachment, UDR will proceed with a slightly revised project layout which removes all additional encroachments from the RPA".

Information contained in the applicant's own documents therefore clearly demonstrate that the requirement for the "minimum necessary to afford relief" has not been met.

Although the applicant has sought justification for the encroachment in part through the removal of the traffic "slip lanes" on West Braddock Road outside of the project site, the actual new encroachment in the RPA constitutes only the new building area. The existing road encroachment is in a different area constituting a separate encroachment, on public property, and as a public street is allowed in the RPA per Section 13-123 of the EMO without the requirement for an exception approval. A new building encroachment of 4,791 square feet on private property is not allowable in the RPA without an exception approval. In addition, this amount of proposed new encroachment is considered an extremely large increase in impervious area and a large loss of RPA buffer. Finally, the removal of impervious surface associated with the "slip lanes" constitutes a separate, allowable encroachment in a different area of the RPA and is not a factor in the approval of a new encroachment when considering the criteria of minimum necessary to afford relief.

Exception Criteria (2). The exception must not be based upon conditions or circumstances that are self-created or self-imposed, nor can the exception arise from conditions or circumstances either permitted or noncomplying that are related to adjacent parcels.

The property owner created the need for the exception by proposing construction within the RPA when there are development options that do not require an encroachment in the RPA. The applicant previously submitted a concept plan to the City with no new encroachments in the RPA clearly demonstrating the proposed layout associated with this request is self-created.

The applicant has stated that the proposed layout is necessary for emergency vehicle access. However, the applicant has not provided any documentation to justify that this is the only acceptable layout for access.

In addition, the adjacent properties are all developed with buildings outside of the RPA. This site is the only one with an existing encroachment in this RPA. Approving a larger RPA encroachment would be inconsistent with the existing conditions of the adjacent parcels.

Exception Criteria (3). Granting the exception must not confer upon the applicant any special privileges that are denied to other property owners in the CBPA overlay district.

This criterion is intended to ensure that an exception request does not give the applicant something that has been denied to others in similar situations. While there exists a very small

number of minor encroachments that have been approved in other nearby local jurisdictions, these are not only inconsistent in scale to this exception request but are also of a different nature. The City has never approved an exception within its CBPA overlay district since the ordinance update in 2004 demonstrating these privileges have been denied to other property owners within the City of Alexandria.

Although no other exception requests have been formally submitted by other property owners in the City of Alexandria, numerous applicants have submitted initial concept plans for review with additional encroachments located within the RPA. It is standard City practice to work with developers during the site plan submittal process to avoid any new encroachments or the need to seek a formal exception. In addition, City practice also encourages the removal of existing encroachments and typically requires their removal and new plantings to help restore the RPA to a natural state.

For example, DSUP2018-00006, Public Storage, initially submitted a site plan utilizing the existing impervious area footprint located on the site for redevelopment. Through the site plan review process staff worked with the applicant to find a more suitable layout for the property that respected the RPA. Ultimately, a plan was approved that stipulates removal all existing impervious area from the RPA, removal all invasive species, replanting the RPA with native vegetation, and dedicating the land to the City.

The applicant has sought to justify the proposed encroachment in their application by asserting that the removal of the slip lanes on West Braddock Road in the RPA constitutes an overall smaller encroachment. However, this is an inaccurate description of a new encroachment. Individual encroachments are separated by area. In this case the new encroachment request is for 4,791 square feet of building area. The slip lanes are a separate, existing encroachment that is not part of the site and is not related to this new encroachment request.

Granting this exception would confer special privileges to the applicant that have been denied to other property owners in the City. In this case, it would set a precedent in the City that could allow for future new encroachments in the RPA.

Exception Criteria (4). The exception must be consistent with the purpose and intent of the overlay district, and not injurious to water quality, the neighborhood or otherwise detrimental to the public welfare.

This exception is not consistent with the intent of the CPBA overlay district defined in Section 13-105. Per Section 13-101 of the Zoning Ordinance, the purpose of the overlay district is to safeguard the waters of the Commonwealth from pollution and to prevent any increase in pollution of state waters. As shown in the submitted WQIA, the development proposes a total of 5,226 square feet of impervious surface in the RPA, an 1,100% increase. Impervious areas are a documented source of stormwater pollution and allowing such a sizable increase in impervious area in a buffer zone that exists to protect water quality would be detrimental to the public welfare.

Applicant has stated that stormwater treatment should be considered in their application. However, all redevelopment projects are required to treat stormwater runoff using stormwater Best Management Practices (BMPs). The proposed BMPs for stormwater treatment are a requirement of the development regardless of whether or not an encroachment is allowed within the RPA.

Exception Criteria (5). Reasonable and appropriate conditions are imposed, as warranted, to prevent the allowed activity from causing degradation of water quality.

This criterion is intended to ensure that conditions are imposed to, among other things, protect water quality and the functionality of an RPA as if it were undisturbed. This criterion must be considered as a whole with the previous four required criteria for approval as all five must be met to meet the approval threshold. In addition, proposed mitigation is not a reason to approve an exception, it is a condition imposed to alleviate the damage to water quality should an exception be found to meet the exception criteria and subsequently be approved. An applicant may propose extensive amounts of mitigation and still not meet the four preceding criteria, resulting in an exception request that does not meet the requirements for approval. Although the applicant has proposed mitigation that would provide a benefit to water quality, it does not justify the addition of a large new encroachment in the RPA, especially given that the request does not satisfy the preceding criteria one through four.

The applicant has submitted several options for mitigation; however, they do not equate to the function of an undisturbed RPA or the loss of RPA buffer. The mitigation options proposed by the applicant are discussed below.

Stream Bank Restoration

The City recently completed the *Phase III Stream Assessment: Stream Restoration and Outfall Stabilization Feasibility Study (February 2019)* to identify area for future restoration. While Strawberry Run and Taylor Run were identified as the top two priority sites and are currently in the design phase, this study did not identify this stream reach as a candidate for restoration.

In response to this exception request, staff worked with the City's consultant to determine if the proposed stream restoration would be feasible and beneficial to perform. The consultant with extensive stream restoration knowledge that include Rosgen techniques of natural channel design visited the site to perform the assessment.

Using the *Recommendations of the Expert Panel to Define Removal Rates for Individual Stream Restoration Projects* (Schueler and Stack, 2014) developed by the Chesapeake Bay Program for restoration projects to achieve Bay pollution reductions and completing a Bank Assessment for Non-point source Consequences of Sediment (BANCS) assessment, the consultant determined there was a potential to reduce approximately 16.5 lbs./year of phosphorous that may be achieved if a stream restoration project. By comparison, the City's Strawberry Run Stream Restoration Project that is currently in design to restore approximately 900 linear feet of stream is slated to remove over 340 lbs./year of phosphorus using the BANCS assessment when completed.

There are numerous site and anticipated engineering and construction constraints that could affect the feasibility of a stream restoration or the costs for design and construction that includes the following: (1) given the steep banks and location of North Beauregard Street, access to the stream is limited and would require lane closures and would preclude construction of a suitable floodplain for natural channel restoration and (2) removal of individual mature trees and their critical root zones would likely compromise the adjacent road slope stability.

Finally, the estimated cost of a stream restoration for this reach far exceeds a cost benefit ratio that would be considered for a stream restoration project. The City's consultant provided an anticipated design and construction cost of \$1.78 million to restore this 850 linear foot stretch. This estimate is somewhat consistent with other stream restoration projects in the City; however, the site constraints would increase the cost estimate greatly. Therefore, the City would consider this cost to likely be lower than a cost estimate developed after the completion of a feasibility study and concept design using the City's CIP project estimating approach. Given that the project may reduce approximately 16.5 lbs./year of phosphorus, the cost per pound would be nearly \$108,000 to remove one pound of phosphorus. The Virginia Department of Environmental Quality (VDEQ) has a threshold of \$50,000 per pound of phosphorus to even consider a project. Additionally, the City's Strawberry Run Stream Restoration Project (which includes an \$800,000 state matching grant for a total project cost of \$1,600,000) is projected to remove 340 lbs./year of phosphorus at a cost of just over \$5,000 to remove one pound of phosphorus. UDR's exception request letter (November 13, 2019) commits \$800,000 for a "stream bank restoration" while Exhibit C of the letter proposed \$500,000 which is inconsistent. Either way, this estimate is less than half of a reasonable cost estimate for such a project. Mitigation in another location of the City, in a different local watershed, does not equate to mitigation for disturbance to the RPA as proposed for this project. Mitigation should occur in for the proximate RPA to prevent degradation of water quality in the protected stream reach.

Given the issues with feasibility, site constraints, and cost; and the previous study conducted by the City that did not identify this location as a candidate for stream restoration, the City does not feel that this stream is a suitable candidate for a stream restoration.

Reforestation

The proposed reforestation does not consider that the project proposes to remove 118 of 183 existing mature trees currently on the site. The area proposed for reforestation would be planted with seedlings, many of which will not survive, and is much further away from the RPA than the mature trees that would be removed if this exception is granted with the development. The phosphorous removal gained from any replanting will be negated by the additional phosphorous generated by the loss of 118 mature trees on the site.

Pet Waste Plan

The option to create pet waste plans are not considered mitigation for RPAs. The overwhelming majority of multifamily residential properties contain pet waste mitigation regardless of location in keeping with best practices to support cleanliness for the private property. In addition, the City cannot count any private pet waste plans for meeting any permit requirements.

Removing Slip Lanes

Finally, the applicant has included the removal of slip lanes at West Braddock Road and N. Beauregard as mitigation for the proposed RPA encroachment. However, T&ES Transportation Planning staff intends to require the removal of the slip lanes for traffic planning purposes regardless of whether the RPA encroachment is approved. There is no additional benefit beyond what would be proposed without the encroachment for the loss of the slip lanes.

The proposed mitigation options do not negate the permanent loss of additional buffer area in the RPA and the introduction of new impervious surface that would negatively impact water quality. The adjacent perennial stream is not suitable for stream restoration, the reforestation does not add additional benefit beyond mitigating what will be lost from the removal of the existing mature trees, the pet waste plan is not a valid buffer mitigation option, and the slip lanes will be required to be removed even if the encroachment is denied.

IV. CONCLUSION

In conclusion, in order to grant an exception, the request must meet all five exception criteria. Staff has evaluated this exception request and has found that it does not meet any of the exception criteria listed in the ordinance, let alone all five criteria as required for approval. Contrary to the applicant's assertion, the request for exception must meet these criteria irrespective of proposed mitigation measures, as these measures would only be applicable if the request first meets the criteria as required. As a result, when evaluating this request against the required exception criteria in the EMO, the application does not meet the criteria by a preponderance of the evidence as required under the Zoning Ordinance in order to be granted. Therefore, this exception should be denied.

Further, the approval of this exception would be in conflict with the principles of the Eco-City Alexandria Charter and Environmental Action Plan. In addition, allowing such a large new encroachment in the RPA that fails to meet the criteria for approval sets a precedent in the City that would make it difficult to deny future exception requests.

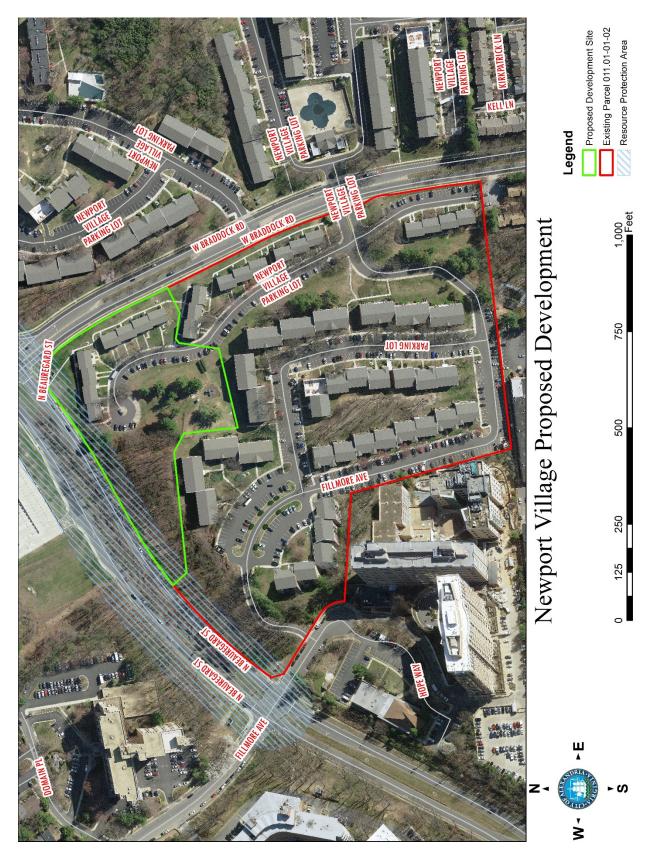
V. STAFF RECOMMENDATIONS

Staff recommend **denial** of the exception request.

VI. ATTACHMENTS

Attachment 1: Exception Request Letter <u>Attachment 2:</u> Encroachment and Mitigation Exhibit <u>Attachment 3:</u> Mitigation Chart <u>Attachment 4:</u> Water Quality Impact Assessment <u>Attachment 5:</u> Existing Conditions <u>Attachment 6:</u> Select Pages from the Concept 2 Site Plan <u>Attachment 7:</u> Original Exception Request Letter dated September 5th <u>Attachment 8:</u> Concept 2 plan revised

Newport Village RPA Exception Request 4898 West Braddock Road



ATTACHMENT #1



Kenneth W. Wire kwire@wiregill.com 703-677-3129

November 16, 2019

VIA EMAIL TO yon.lambert@alexandriava.gov

Yon Lambert, Director Department of Transportation and Environmental Services 301 King Street, Suite 4100 Alexandria, Virginia 22314

RE: Newport Village 4898 W. Braddock Road (the "Property")

Dear Mr. Lambert,

On behalf of my client, UDR, I am submitting this hearing request for approval of an encroachment into the Resource Protection Area ("RPA") along the eastern side of Beauregard Street for the above referenced property. UDR requests this exception to Zoning Ordinance Section 13-107 as permitted by Zoning Ordinance Section 13-119. As shown on the attached exhibits, the total improvements to the Property will increase the impervious area within the RPA with offsets proposed by removing City impervious area associated with travel lanes at this intersection and replanting other areas. This exception is appropriate because 1) the Property's extreme topography precludes any other viable redevelopment scenario; 2) the existing RPA is separated from the stream itself by several lanes of Beauregard Street; 3) the overall water quality will be significantly improved by the proposed mitigation; and 4) the proposed building line will create a better interaction between the building façade and the future BRT station.

I. <u>Project Background</u>

UDR is the owner of the Newport Village Apartments (the "Property") and has worked with City staff over the past several months to determine a viable redevelopment strategy for a portion of the Property located at the south western intersection of Beauregard Street and Braddock Road. UDR's initial concept was to demolish the three smaller scale buildings located at this intersection and redevelop their footprint with a larger multi-family building with below grade parking. One of these buildings currently encroaches in the RPA in the same location of the requested exception. *See* Existing Conditions, *attached* as Exhibit A.

As you may know, the grade of the Property is very steep as it rises significantly to the south. During the initial design phase, the UDR team obtained updated construction pricing associated with pushing the new building into the grade which proved to be extremely inefficient and costly. UDR and its team spent the past few months redesigning the new development

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proposal to move the new building out from the steep slope and save one of the existing buildings by revisiting the proposed project's footprint. *See* Proposed Plan, *attached* as Exhibit B.

UDR and its team worked with the City to also design a better interaction of the new building with the proposed BRT stop at Beauregard Street and orient the active uses within the building towards the BRT stop. The team also proposes shifting a portion of the building slightly further into the RPA to enhance the pedestrian experience at the intersection and remove the City's existing impervious slip lanes. UDR is supportive of this approach and requests a positive EPC recommendation for the encroachment.

II. <u>Existing RPA Conditions</u>

The proposed redevelopment site is located at the south-west corner of N. Beauregard Street and W. Braddock Road. There is an RPA associated with the stream that runs in the Beauregard median. The stream in the RPA is in poor condition with heavily eroding slopes and invasive species. The RPA extends between 39 feet and 59 feet into the redevelopment site on the western side. There are existing improvements located within the RPA including the N. Beauregard travel lanes, sidewalks and a very small corner of the northwest building. Proposed within the redevelopment site, there is a portion of a building and its associated concrete walkways located within the RPA. UDR and its team recently field verified the RPA which begins at the top of bank for the stream in the middle of Beauregard. The total on-site impervious area currently within the RPA is approximately 5,135 s.f. (0.12 Ac). *See* Exhibit A.

III. <u>Required vs. Proposed RPA Mitigation</u>

The proposed redevelopment consists of the demolition of two existing buildings, associated walkways/sidewalks, parking, drive aisles and utilities, and the construction of a new multifamily building with associated walkways, drive aisles and utilities. If the RPA did not exist, the proposed building would comply with current stormwater requirements and no additional mitigation. The attached Chart demonstrates the difference between the no additional mitigation scenario and the proposed additional mitigations that would be provided if this exception is granted. *See* Chart, *attached* as Exhibit C. We have also attached a Water Quality Impact Assessment Report ("WQIA"), *attached* as Exhibit D.

Section IV below, addresses each of the factors that must be addressed granting any RPA exception. We note, however, that the very purpose of the RPA regulations is to protect water quality within the Chesapeake Bay Watershed. Exhibit A shows that there would be approximately 5,135 s.f. (0.12 Ac) of existing impervious area that will be removed from the RPA for the proposed improvements. Approximately 5,100 s.f. of the proposed building and associated walkways are proposed within the RPA. Therefore, the total on-site impervious area proposed within the RPA would be shifted landward and *reduced* by 35 s.f.

In order to mitigate the impacts of the proposed improvements within the RPA, the owner is proposing the following (*See Chart and summary* attached Exhibit C):

1. **Stream Bank Stabilization**—The stabilization of the stream bank of 800 linear feet is estimated to allow for removal of at least 24.5 lbs. of phosphorus, which far exceeds any other mitigation strategy, and alone mitigates the impacts of the proposed development by 10-fold.

Specifically, the stream bank stabilization/restoration includes removal of debris and invasive plants/trees; the implementation of imbricated boulders; and in-stream step pools to address high velocities, bed and bank shear stress, and erosion. Pursuant to Virginia DEQ Guidance Memo No 15-2005 interim rates, after implementing these practices, the estimated pollutant of concern ("POC") reduction credit may be up to 30.6 pounds of TP, 33.75 pounds of TN, and 3,656 pounds of TSS. However, to be conservative we are proposing a lower estimate of 24.5 pounds of TP, 23.16 lbs of TN and 2,509 pounds of TSS. Credit for POC's is determined after a Bank Assessment for Non-Point Consequences of Sediment (BANCS) has been completed and verified by DEQ. UDR agrees to participate in the DEQ verification process.

- 2. **Removal of the existing slip lanes at the intersection of North Beauregard and West Braddock** In order to reduce the impervious surface within the RPA, the two existing slip lanes at the intersection will be removed. The removal of the existing slip lanes will remove approximately 4,500 s.f. (0.10 Ac) of impervious area from the RPA. With the removal of the slip lanes, vehicular contact areas will be removed from the RPA and replaced with pervious cover. This will reduce the amount of high-concentration pollutants (0.14 lbs of phosphorous) generated by surface runoff being conveyed into the RPA.
- 3. **Reforestation** Reforestation using native species will be provided within the RPA (approximately .40 Ac or 17,420 s.f. is proposed). 0.15 lbs of phosphorus will be removed in implementing this strategy.
- 4. **Pet Waste Plan** UDR is also proposing a pet waste management system to reduce pet waste intrusion into the water shed.

With each of the above mitigation measures, <u>the granting of this exception conditioned</u> on the implementation of each of these measures will improve water quality by **27.54 lbs of total phosphorus removal per year** or 24.8 lbs of phosphorus above the baseline amount that would otherwise be achieved by not granting this exception.

IV. <u>Section 13-119 – Exceptions</u>

This proposed exception will greatly improve water quality and is eligible for the exception under Section 13-119. Under Section 13-119 – Exceptions, the Director of T&ES shall review the request for an exception and may grant the exception with such conditions and safeguards as deemed necessary to further the purpose and intent of this Article XIII "Environmental Management" if the Director of T&ES finds that an applicant has demonstrated by a preponderance of the evidence that the following criteria are met. UDR's response to each criterion is stated below in italics.

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(1) Granting the exception will not confer upon the applicant any special privileges that are denied to other property owners in the CBPA overlay district;

Granting an exception in this case would not confer special privileges to UDR that have previously been denied to other property owners primarily because the proposed amount of encroachment in the RPA is slightly reduced from the existing encroachment of the building and slip lanes and the proposal includes a significant amount of mitigations that other property owners likely have not been willing to provide. Additionally, UDR's project is different from other properties and developments because of its unique characteristics inherent to the Property and the imposition of more onerous conditions to greatly improve water quality than would be possible by not granting the exception.

Unique characteristics of the Property include the steep topography, the urbanized setting whereby the stream is in the center of a four-lane major thoroughfare, and the removal of existing impervious area (5,135 s.f.) that currently encroaches within the RPA boundary.

Unique elements of the proposed development plan include: mitigations that will increase water quality beyond the quality that exists; the limited size of the proposed development within the RPA; and in total, the project will add significant levels of housing located adjacent to the City's BRT, the use of which will reduce single-occupancy vehicle trips (SOVs).

(2) The exception is not based upon conditions or circumstances that are self-created or self-imposed, nor does the exception arise from conditions or circumstances either permitted or noncomplying that are related to adjacent parcels;

The steep topography of the Property and limits on reasonable development costs of construction type are conditions and circumstances that are not self-created. After performing feasibility analysis, it has been determined that no other plan is feasible. Additionally, this exception does not arise from conditions or circumstances permitted or noncomplying that are related to adjacent parcels.

(3) The exception is the minimum necessary to afford relief;

The proposed development within the RPA is 5,100 s.f. and the existing impervious area that will be removed from the RPA is 5,135 s.f. UDR's proposed project clearly makes the existing condition no worse, and in fact is slightly better since 35 s.f. smaller. UDR and their design team attempted other designs that are not realistically feasible considering the steep topography. The area proposed to develop is in fact the minimum area to develop a building, given the constraints of topography, building height limits of the construction type and retention of the existing buildings to remain on the Property. Any other alternative plan is not reasonably feasible given the unique characteristics of the Property, namely the topography whereby pushing the building into the grade exponentially increases the construction costs and requires demolition of existing buildings to remain. November 16, 2019 Page 5 of 5

The proposed encroachment is also only for a portion of the building fronting on Beauregard. Of the 395 linear feet of building frontage on Beauregard only 197 linear feet is requested to encroach into the RPA.

(4) The exception will be consistent with the purpose and intent of the overlay district, and not injurious to water quality, the neighborhood or otherwise detrimental to the public welfare;

UDR has committed to mitigations stated above which include stream bank restoration, valued at \$500,000, and removes at least 24.5 lbs. of phosphorus alone; removal of impervious area of the existing slip lanes; and reforestation—all of which are consistent with the purpose and intent of the overlay district. As stated in 13-1010, the purpose of the overlay zone is: minimizing the potential pollution from stormwater runoff and erosion and sedimentation; the reduction of the introduction of harmful nutrients and toxins into state waters; and the maximization of rainwater infiltration while protecting groundwater. With the proposed mitigations, the development will be consistent with the ultimate policy goal of increasing the water quality beyond its existing levels, which promotes the general welfare.

(5) Reasonable and appropriate conditions are imposed, as warranted, to prevent the allowed activity from causing degradation of water quality.

UDR agrees to reasonable and appropriate conditions including the proposed mitigations and future monitoring, in order to prevent the proposed development from causing degradation of water quality.

For each of these reasons, we respectfully request a hearing by the Environmental Policy Committee for an exception to Zoning Ordinance Section 13-107 for the proposed encroachments and improvements within the RPA.

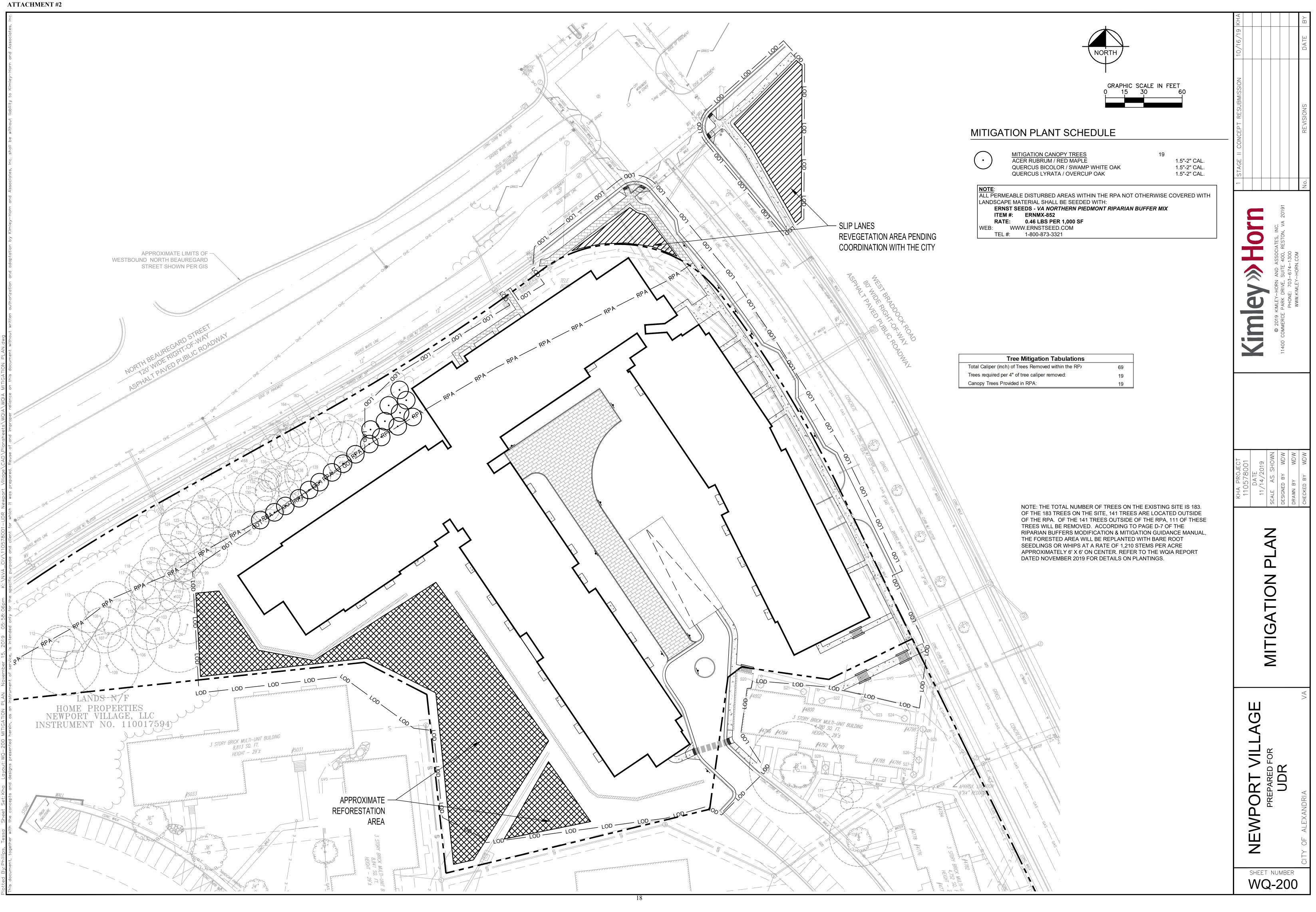
Sincerely,

Kenneth W. Wire

Enclosures

CC: Praveen Kathpal, Chair, Environmental Policy Commission Karl Moritz, Director, Department of Planning and Zoning

4842-8514-9356, v. 1



ATTACHMENT #3

Exhibit C - RPA Water Quality Additional Mitigation Chart and Summary

	EXISTING	REQUIRED	PROPOSED	<u>% OVER</u> REQUIREMENTS
OPEN SPACE	75%	40%	54%	34%
VEGETATED CANOPY COVERAGE (Does not include reforestation proposal below)	35%	25% of Site	25%	0%
STORMWATER MANAGEMENT				
Stormwater outfall volume (10YR Dischage Volume)	22.01	22.01	21.16	4%
SWM Best Management Practice Summary*		2.75 lbs TP	2.81 lbs TP*	
Permeable Pavement			0.37 lbs TP	
Urban Biorentention Vegetated Roof			1.23 lbs TP 0.35 lbs TP	
DC Sand Filter			0.33 IDS TP 0.71 Ibs TP	
Filtering Device			0.17 lbs TP	
BMP Subtotal*		2.75 lbs TP	2.81 lbs TP*	2%
*Based on conceptual level SWM design, subject to char	ige pending fin	al design. Minimum requirement		
EXCESS TMDL REMOVAL STRATEGIES				
REFORESTATION	N/A	Not Currently Required	Up to 0.40 Acres On-Site. The phosphorus removal has been calculated based on the Chesapeake Bay TMDL Guidance - Table V.H.1 – Land Use Change Conversion Efficiency Table for pervious land converted to forested.	Please see table below
STREAM BANK STABILIZATION PROJECT	ΝΆ	The stream bank stabilization/restoration may include remov construction debris, invasive plants/trees and the implementati imbricated boulders, and in-stream step pools to address velocities, bed and bank shear stress and erosion. Using the ir rates from the Virginia DEQ Guidance Memo No 15-2005 estimated pollutant of concern (POC) reduction credit implementing these practices may be up to 30.6 pounds of TP, pounds of TN and 3,656 pounds of TSS. Credit for these PO determined after a Bank Assessment for Non-Point Consequenc Sediment (BANCS) has been completed and verified by DEQ. the total amount of credit is uncertain, UDR is proposing a tar 24.5 lbs of TP removal. UDR proposes 800 linear feet of stream bank stabilization of alternate of \$500,000 towards TMDL Credits.		
SLIP LANE REMOVAL AND REPLACEMENT WITH PERVIOUS PAVERS AND OPEN SPACE	N∕A	Not Currently Required	Removal of 0.1 acres of high vehicle contact areas and shifting of impervious areas landward. Replacing the impervious concrete with pervious pavement and plantings will reduce pollutants and will contribute towards City of Alexandria's reduced TMDL goals. The phosphorus removal has been calculated based on the Chesapeake Bay TMDL Guidance - Table V.H.1 – Land Use Change Conversion Efficiency Table for impervious land converted to pervious.	Please see table below
PET WASTE PILOT PROGRAM FOR E. COLI MNGMT	ΝΆ	Not Currently Required	The unnamed tributary of Lucky Run is part of the Four Mile Run Watershed which is subject to a Bacteria TMDL. Additionally, the Pet Waste Pilot Program will help the City accomplish their "Minimum Control Measure #1" for their CBPA TMDL Plan. This pilot program will be implemented in all of UDR's Newport Vilage including the proposed new building and the existing, surrounding multi-family community. The Owner pledges to do the following as part of this program: •Develop written materials (pamphlets) and distribute to tenants •Install pet waste stations and use social media to document and promote •Agree to be part of an educational, speaking engagement •Pilot requiring residents to register their pets in a DNA database and then fine violators who do not clean up pet waste.	N⁄A

TMDL IMPACT OF PROPOSED WATER QUALITY IMPROVEME	TOTAL PHOSPHORUS REDUCTION (LB/YR)		TOTAL SUSPENDED SOLIDS REMOVAL (LB/YR)
Required Stormwater Management	2.75	15.27	1,290.30
Reforestation	0.15	1.04	70.38
Streambank Stabilization Project	24.50	23.16	2,509.02
Land Use Change - Slip Lane	0.14	0.97	65.69
Total	27.54	40.43	3,935.39
% over requirement	1001%	265%	305%

Newport Village Phase II Water Quality Impact Assessment



Prepared By:

Kimley»Horn

November 2019

WATER QUALITY IMPACT ASSESSMENT NEWPORT VILLAGE PHASE II KIMLEY-HORN & ASSOCIATES, INC. NOVEMBER 2019

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LIST OF ATTACHMENTS

- 1. ESA Environmental Site Assessment
- 2. WQ-100 Existing Vegetation Conditions
- 3. WQ-110 Existing Vegetation Data
- 4. WQ-200 Proposed Mitigation Plan
- 5. WQIA Checklist Ordinance 13-117 Checklist
- 6. Comment Response Document
- 7. Selected sheets from Phase 2 Concept Plan Set

1.0 Introduction

This water quality impact assessment (WQIA) has been prepared for the redevelopment of a 4.19-acre parcel within the Newport Village Development (Site) located along the southern side of Beauregard Street and the western side of Braddock Road in the City of Alexandria, Virginia. The owner/operator entity is Newport Village, LLC. (UDR) and currently consists of garden style apartments, is proposed to redevelop into higher density apartment complex.

The intent of this WQIA is to identify major impacts for development or redevelopment within resource protection areas (RPA), or under an exception that involves more than 5,000 square feet of land disturbance within an RPA or for any development or redevelopment within a resource management area (RMA) which involves more than 5,000 square feet of land disturbance adjacent to an RPA. The proposed redevelopment within the 4.19-acre Newport Village parcel will encroach into the RPA and more than 5,000 square feet of disturbance is proposed adjacent to the RPA within the RMA. This WQIA will show how the proposed redevelopment will impact the RPA and adjacent RMA.

The existing and proposed use will not change from multi family residential and the use with the RPA will remain forested/pervious. The RPA on site (and off site) has already been impacted by the development of Beauregard Street and the existing Newport Village development. The proposed use will disturb 0.36 acres within the RPA and will add 0.11 acres of impervious area for a total of 0.12 impervious acres within the RPA post redevelopment (see Table 8 Land Cover Change for an land cover area summary). The areas within the RPA that will be impacted from the proposed use/redevelopment are will be treated with water quality BMPs to prevent further impact to the RPA and its environmentally sensitive areas.

The proposed redevelopment will fully meet water quality requirements through implementation of seven (7) BMPs. Newport Village, LLC. (UDR) will provide additional mitigation options to enhance water quality in the City. These other options are discussed in Section 4.

2.0 Existing Conditions

The existing Site is 4.19 acres and is located southwest of the intersection of Beauregard Street and Braddock Road. An unnamed, perennial, tributary to Lucky Run is located north of the Site, in Beauregard Street's median and flows west to east. Beauregard Street, a two (2) lane road and its sidewalks, run west to east in the unnamed tributaries RPA. The impervious area from Beauregard Street contributes significant uncontrolled stormwater runoff to the unnamed tributary of Lucky Run. The Site itself consists of garden style apartments, an access road and parking lot, sidewalks, and apartment amenities. The RPA that does come onto the Site is approximately 0.70 acres and consists of 0.01 acres of impervious area, 0.22 acres of pervious area and 0.47 acres of forested area. Table 1 summarizes the RPA land cover on site. Below is more information regarding the existing conditions for the Site.

2.1 Environmental Site Assessment

The Environmental Site Assessment (ESA) was completed in accordance with 13-112 of the City Code. Approximately 2.36 acres of the site contains slopes 15% or more. Of the 2.36 acres approximately 0.50 acres of the site is located within the RPA and contains slopes 15% or more. The following items are not present on the Site.

- No intermittent streams contained with a natural channel.
- No highly erodible (K factor >0.28) and highly permeable soils as available from existing public documents
- No known areas of contamination
- No springs, seeps, and related features
- No wetland permits are required
- No dredging or disposal of dredging materials is proposed as part of this project within the project limits or outside of the project limits
- No impacts on adjacent shellfish bed, submerged aquatic vegetation and fish spawning areas are expected as part of this project

A wetland delineation for the adjacent perennial stream was performed consistent with current procedures promulgated by the U.S. Army Corps of Engineers and the Environmental Protection Agency. The site-specific evaluation of the RPA boundary was determined from this wetland delineation. See the ESA sheet for location of the above referenced items.

2.2 Land Cover

The existing Site consists of 0.99 acres of impervious area (building, road, sidewalk), 1.73 acres of pervious area (grass) and 1.47 acres of forested area. The landcover within the RPA on the existing Site consists of 0.01 acres of impervious area, 0.22 acres of pervious area and 0.47 acres of forested area. Table 1 provides a summary below and Sheet C-100 shows the existing site conditions and land cover in the RPA.

Table 1 Existing Land Cover Summary			
Existing Land Cover on Site Existing Land Cover in RPA			
Land Cover	Area (Acres)	Area (Acres)	
Impervious	0.99	0.01	
Pervious	1.73	0.22	
Forest 1.47		0.47	
Total 4.19		0.70	

2.2 Topography

The topography within the 4.19-acre site varies from 1% slope to the 50%+ slope. The existing developed area consisting of approximately 1.5 acres generally does not exceed 15% slope. The existing buildings are designed with apartments that walk out onto Beauregard and Braddock. To allow for this the buildings have been graded with a 15%+ slope. Approximately 2.36 acres of the Site has a slope of 15%+ including most of the RPA.

2.3 Hydrology

An unnamed perennial tributary to Lucky Run is located north of the Site within Beauregard Street's median and flows southwest to northeast. The perennial stream flows into a 48" RCP adjacent to the Beauregard and Braddock intersection and continues downstream to Lucky Run. This stream is the source of the RPA that extends onto the Site. Approximately 0.59 acres of RPA south of this stream consists of road and sidewalk (Beauregard Street) and 0.70 acres extending onto the northern side of the Site. Sheet C-400 shows the land cover located in the RPA that extends on Site.

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The site is divided into two (2) points of analysis (POA). The first POA is located within an island southeast of the intersection of Beauregard and Braddock and drains approximately 2.97 acres, has an 86 curve number (CN) and a time of concentration (TOC) of 5 minutes. An existing 18" RCP and an existing 15" tie into an existing 15" (upsized to 24"), running southeast to northwest and located on the eastern side of Braddock Road. The second POA is located at the 48" concrete headwall conveying the existing perennial stream and drains approximately 1.22 acres, has a 79-curve number (CN) and a time of concentration (TOC) of 9.9 minutes. POA #2 drains to the existing 48" concrete headwall through an existing yard inlet located along Beauregard Street. The yard inlet captures flow generated from the western portion of the site and drains it through an existing 18" RCP that flows south to north under Beauregard Street. Sheet C-401 through C-404 provides hydrology information for the existing site.

2.4 Pollutant Loads

Using the Virginia Runoff Reduction Method (VRRM) new development spreadsheet the estimated uncontrolled pollutant load generated from the existing Site is 1.58 lbs. of phosphorus

2.5 BMPs

No existing water quantity or water quality BMPs are located within the Site to attenuate peak flows and treat stormwater runoff generated from the existing Site. Storm runoff is collected onsite and discharge through RCP to the adjacent unnamed tributary to Lucky Run.

2.6 Soils

The NRCS web soil survey shows two (2) soils on the Site: Sassafras-Marumsco complex, 7 to 15 percent slopes (91C) and the Urban land-Kingstowne complex (100).

The Sassafras portion of the Sassafras-Marumsco complex (91C) generally consists of slopes that are 7 to 15 percent. This component is on terraces, coastal plains. The parent material consists of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches (or restricted depth) is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 3e. This soil does not meet hydric criteria.

The Marumsco portion of the Sassafras-Marumsco complex (91C) generally consists of slopes that are 7 to 15 percent. This component is on terraces on coastal plains. The parent material consists of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches (or restricted depth) is moderate. Shrink-swell potential is high. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 15 inches during January, February, March, November, December. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 3w. This soil does not meet hydric criteria.

The Urban Land portion of the Kingstowne complex (100) generally consists of miscellaneous soils. The Kingstowne portion of the Kingstowne complex (100). Generally consists of slopes that are 0 to 45 percent. This component is on marine terraces on coastal plains. The parent material consists of Earthy fill of fluviomarine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth

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of 60 inches (or restricted depth) is moderate. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 40 inches during January, February, March, April, May, November, December. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

A soils report completed by ECS Mid Atlantic, LLC. On December 7, 2018 indicates the presences of marine clays underlying, manmade fill and naturally occurring alluvial soils and consisted of gravel, sand, silt and clay. The soil report indicated that there are slope stability issues on site. Mitigation measures were provided by the geotechnical engineer and will be incorporated into the proposed redevelopment plans. Per the Virginia Department of Environmental Quality (DEQ) VEGIS web maps, accessed on August 13, 2018, no on-site contamination is known.

2.7 Geology

According to the Geologic Atlas of the City of Alexandria, Virginia the underlying geology within the site is Lincolnia Silty Clay. Lincolnia Silty Clay can generally be described as massive to slabby-bedded silty clay and clayey silt, moderately sandy in many places. Color typically light green-gray or bluegray where fresh and red-brown where weathered; commonly mottled near the water table. Clay fraction dominated by expandable lattice types. Small to medium sized lenses and channel-like bodies of fine to medium arkosic sand are common near contacts with the Winkler and Cameron Valley sand members. Thickness typically 50-60 feet, but may exceed 100 feet beneath parts of Lincolnia. Fine, wavy laminations and sandy partings occasionally present in slabby-bedded units. Probably deposited as overbank sediment on a broad, stable floodplain. The Lincolnia silty clay appears to be in a large scale lateral facies relationship with the upper part of the Cameron Valley sand (Kpcv, below). The Barcroft diamicton (Kpb) occurs discontinuously at or near the base of the Lincolnia silty clay. It contains pebbles, cobbles, and boulders up to 18 inches long embedded in a dense, massive to crudely layered, red-brown to green-gray clayey to loamy matrix with incipient soil horizonation and organic layers. Clasts include vein quartz, sandstone, and skolithos-bearing quartzites; some are faceted, pitted, and resemble ventifacts. Maximum observed thickness is about 18 feet

2.8 Vegetation

Kimley Horn performed a tree survey to identify trees 6" and greater in diameter at breast height (DBH) and located a total of 183 trees on the Site including 42 trees located in the RPA. 141 trees are not located in the RPA. Kimley Horn also identified 28 shrubs. None of those shrubs are located in the RPA. The existing trees and shrubs consist of native hardwood typically found in the mid-Atlantic region and include Pine, Oak, Maple, Cherry, Redbud, and Birch. Approximately 19 of the 183 trees were dead and or dying including one (1) tree dead or dying tree in the RPA. Five (5) trees were 30" or more DBH. Approximately 135 trees are 18" DBH or lower and approximately 70% of the species found were oak. Most of the existing vegetation is located in the Site's northwestern corner and western boundary. The existing vegetation contributes approximately 0.47 acres of forested canopy in the RPA. The existing vegetation is shown on sheet WQ-100 and summarized on WQ-110 and species are summarized in Table 2 below.

Table 2					
Existing	Existing Vegetation Species				
Species	Quantity	Percentage			
Maple	22	12.02%			
Birch	1	0.55%			
Redbud	2	1.09%			
Hawthorn	2	1.09%			
Crape Myrtle	7	3.83%			
Pine	15	8.20%			
Cherry	3	1.64%			
Oak	129	70.49%			
Locust	2	1.09%			
Total	183	100.00%			

Approximately 118 of the 183 existing trees and 19 of the existing 28 shrubs will be removed including seven (7) trees in the RPA. No existing shrubs in the RPA will be removed as part of the proposed development. The total caliper for the trees removed in the RPA is 69" (see Table 6 for each tree's caliper) and the total canopy area removed in the RPA is 2,650 square feet (0.06 acres). Approximately 84 trees are 18" DBH or lower and approximately 66% of the species proposed to be removed are oak. The trees and shrubs proposed to be removed are shown on sheet WQ-100 and summarized on sheet WQ-11 species are summarized in Table 3 below.

Table 3 Existing Vegetation Species Removed				
Species	Quantity	Percentage	Located in RPA	% in RPA
Maple	12	10.17%		
Birch	1	0.85%		
Redbud	2	1.69%		
Hawthorn	2	1.69%	2	29%
Crape Myrtle	4	3.39%	3	43%
Pine	15	12.71%	1	14%
Cherry	2	1.69%		
Oak	78	66.10%	1	14%
Locust	2	1.69%		
Total	118	100.00%	7	100.00%

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Given the existing site and the existing vegetation on the proposed redevelopment will preserve to the greatest extent possible any significant trees and vegetation on the Site and will provide maximum erosion and overland flow benefits from vegetation.

2.9 Sanitary Sewer

An existing 10" gravity sanitary sewer main flows southeast to northwest parallel to Braddock Road and provides service to the existing 4.19-acre Site. The existing sanitary sewer peak flow is 0.38 MGPD and the total average daily flow is 0.095 MGPD. No septic tanks or drain fields are located on the Site. The existing sanitary sewer is shown on sheet C-100.

3.0 Proposed Redevelopment

The proposed redevelopment consists of the construction of high-density multifamily apartments, parking, sidewalks, roads, retaining walls, and stormwater BMPs. Approximately 3.87 acres of the 4.19-acre (92%) Site will be disturbed during the construction including 0.36 acres in the RPA (9% of total land disturbance). Disturbance on site and in RPA primarily consists of earthwork, redevelopment and tree removal. As shown in Table 4, a total of 0.12 impervious acres will be located in the RPA after redevelopment has been completed. To address water quality during this land disturbance, erosion and sediment control devices will be installed to prevent sediment from escaping onto any impervious surfaces or into the storm drains. The post construction BMPs have been designed to treat stormwater runoff before being discharged to an adequate outfall. Below is more information regarding the anticipated proposed conditions for the site.

3.1 Land Cover

The proposed Site consists of approximately 4.19 acres of high-density apartments with 2.23 acres consisting of impervious area (building, road, sidewalk), 1.06 acres consisting of pervious area (grass) and 0.90 acres consisting of forested area. Table 4 provides a summary of the proposed land cover.

Table 4 Proposed Land Cover			
Proposed Land Cover in Proposed Land Cover RPA			
Land Cover Area (Acres)		Area (Acres)	
Impervious	2.23	0.12	
Pervious	1.06	0.08	
Forest	0.9	0.50	
Total	4.19	0.70	

3.2 Topography

The proposed topography within the 4.19-acre Site will continue to vary from 1% slope to the 50%+ slope. The proposed development is about 2.5 acres and will not exceed 5% slope. The new building will extend into the 15%+ slope located on the western side of the Site. Constructing these buildings on the western side of the site will require excavation of the slope and installation 10' - 20' retaining walls along the western property boundaries. The topography in the north western portion of the Site will be preserved to the maximum extent practical to preserve the 15%+ slopes. Slopes 10% to 15%+ will be graded around the buildings where needed to adequately tie in grades. Sheet C-300 shows the proposed grading on the Site.

3.3 Hydrology

The proposed Site will be divided into five (5) drainage areas (DA). DA-A drains 0.17 acres has a CN of 98 and a TOC of 5 minutes. It will tie into the existing Yard Inlet and underlying 18" RCP located at the northern portion of the Site at along Beauregard Street that eventually drains to POA#2 discussed in the existing hydrology section. DA-B drains 0.29 acres, has a CN of 98 and a TOC of 5 minutes. DA-C drains 0.40 acres has a CN of 98 and a TOC of 5 minutes. DA-C drains 0.40 acres has a CN of 98 and a TOC of 5 minutes. DA-D drains 1.59 acres has a CN of 98 and a TOC of 5 minutes. DA-E drains 0.53 acres has a CN of 98 and a TOC of 5 minutes. DA-A, DA-C through DA-E is collected via yard inlets, drop inlets and underdrains and ties into the proposed 15" RCP south of the Site and drains to the existing 15" RCP across Braddock Road and eventually POA #1 discussed in the existing hydrology section. The proposed development and storm drain design will not impact Waters of the United States (WOUS), will not result in significant degradation of water quality that could adversely affect aquatic vegetation or life, and the propose development will not result in significant disruption of hydrology on the site. Attachment C-401 shows hydrology information for the proposed site.

3.4 Pollutant Loads

Using the Virginia Runoff Reduction Method (VRRM) redevelopment spreadsheet the estimated treatment provided through the implementation of the BMPs on the Site is 2.81 lbs of phosphorus. This exceeds the required treatment loads by 0.06 pounds of phosphorus. Sheet C-406 shows the provided BMP credit.

3.5 Proposed BMPs

All BMPs proposed for this Site will be designed in accordance with the Virginia Department of Environmental Quality (DEQ) BMP Clearinghouse standards and specifications. BMPs have been designed to effectively treat water generated from the 4.19-acre Site and tie into the proposed storm sewer infrastructure and eventually tie into the existing storm sewer infrastructure and analyzed to POA #1 and POA #2. The proposed BMP design will not impact WOUS, will not result in significant degradation of water quality that could adversely affect aquatic vegetation or life, and the proposed development will not result in significant disruption of hydrology on the site.

A conceptual level stormwater management design has been provided with this document. As shown on the Virginia Runoff Reduction Methodology (VRRM) spreadsheet on sheets C-405 and C-406, a total of 0.84 acres of pervious area and 2.14 acres of impervious area is being treated, providing a total phosphorus reduction of 2.81 lbs. Table 5, shown below, summarizes the current SWM BMP plan for each drainage area.

Drainage Area A is providing treatment of 0.17 acres of impervious area through SWM #1 - Vegetated roof, and receiving 0.17 lbs of phosphorus credit. Drainage Area B is providing treatment of 0.29 acres of impervious area through SWM #2 - permeable pavement and receiving 0.37 lbs of phosphorus credit. Drainage Area C is providing treatment of 0.40 acres of impervious area through SWM #3 - Urban bioretention and receiving 0.48 lbs of phosphorus credit. Drainage Area D consists of multiple treatment trains as detailed out on Sheet C-405 and calculated on Sheet C-406. In summary, Drainage Area D is providing treatment of 0.75 acres of impervious area and 0.84 acres of pervious area through SWM #4 - Vegetated roof, SWM #5 - Urban bioretention, SWM #6 – Sand Filter and SWM #7 – Sand Filter and receiving 1.63 lbs of phosphorus credit. Drainage Area E is providing treatment of 0.53 acres of impervious area through SWM #8 - manufactured treatment device (hydrodynamic separator), and receiving 0.17 lbs of phosphorus credit. The total pervious and impervious area treated through these BMPs is 0.84 acres and 2.14 acres respectively. The total pounds of phosphorus removed is 2.81. Stormwater management BMP locations are shown on sheet C-405 and SWM calculations are shown on Sheet C-406.

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	Table 5				
		BMP/SW	M Summar	у	
Drainage Area	SWM #	BMP Type	Pervious Area (AC)	Impervious Area (AC)	Phosphorus Credit (Ibs)
А	1	Vegetated Roof	0	0.17	0.17
В	2	Permeable Pavement	0	0.29	0.37
С	3	Urban Bioretention	0	0.4	0.48
	4	Vegetated Roof	0	0.18	0.18
D	5	Urban Bioretention	0	0.53	0.75
	6&7	Filtering Practice #1 (Sand Filter)	0.84	0.04	0.71
Е	E 8 MTD - Hydrodynamic		0	0.53	0.17
	Total		0.84	2.14	2.81

3.6 Soils

The soils in the western portion of the site may be impacted through earthwork activities. The remaining soils onsite have already been impacted through the existing development. No further impacts to soils within the existing developed area are expected. A surplus of fill material should be available from the cut operations on the western portion of the Site. As a result, any necessary fill material for the remaining portions of the Site will be sourced from those cut operations. Should additional fill material be needed, the contractor will provide a source of all fill material and biological, chemical, and suitability testing should be conducted prior to construction. Cut and fill shall be minimized to maximum extent practical based on the existing site constraints and proposed redevelopment.

3.7 Vegetation

The proposed vegetation for the Site has been developed in accordance with the Riparian Buffers Modification & Mitigation Guidance Manual. The proposed vegetation in the RPA is based upon the tree caliper removed. According to Appendix D-5 a 1.5" - 2" tree is required for every 4" of tree DBH removed in the RPA. A total of 7 trees within the RPA will be removed. The total caliper for these trees is 69" According to Vegetation Replacement Rates on page D-5 the total number of 1.5" - 2" caliper trees to replace the trees removed in the RPA is 19. Table 6 provides a summarizes the proposed vegetation calculations.

	Table 6					
	RPA Tree Re	emoval &	Tree Replacer			
Tree No.	Trees Required Per 4" Caliper Removed					
155	Oak	Yes	9	3		
161	Pine	Yes	19	5		
162	Crape Myrtle	Yes	7	2		
169	Hawthorn	Yes	7	2		
170	Crape Myrtle	Yes	12	3		
171	Hawthorn	Yes	8	2		
172	Crape Myrtle	Yes	7	2		
	Total		19			

The proposed plant schedule shown in Table 7 below provides canopy trees in accordance with the recommendations of the Riparian Buffers Modification & Mitigation Guidance Manual as required to mitigate for the trees removed in the RPA. The replanting schedule is also provided on sheet WQ-200.

Table 7 Planting Schedule								
Vegetation Type	Quantity	Size	Species					
Canopy Trees	19	1.5" - 2" Caliper	Acer rubrum	Red Maple				
			Quercus bicolor	Swamp White Oak				
			Quercus lyrata	Overcup Oak				

In addition to the proposed canopy trees, understory trees and small shrubs, the Virginia Northern Piedmont Riparian Mix is proposed within the disturbed areas of the RPA (0.46 acres including 0.10 acres for slip lanes) to assist in providing native seeding for all the disturbed areas. Approximately 508 bare root seedlings or whips will be planted in accordance with page D-7 of the Riparian Buffers Modification & Mitigation Manual for the proposed 0.42-acre reforestation area. The remaining portions of the Site to be landscaped will be planted in accordance with the City of Alexandria 2019 Landscape Guidelines and will highlight native vegetation that will supplement the RPA buffer in a manner that provides for pollutant removal, erosion, and runoff control. The development will not result in unnecessary destruction of plant material on Site. The proposed vegetation will provide maximum erosion and overland flow benefits possible.

3.8 Sanitary Sewer

The existing 10" sanitary sewer gravity main will remain in place. The proposed structures on the Site will tie into the existing sanitary sewer on the Site approximately one hundred feet (100') south of the southwest corner of the intersection of Beauregard Street and Braddock Road. According to a preliminary sanitary

sewer adequate outfall analysis, the receiving sanitary sewer system has capacity for the proposed Newport Village redevelopment. The sanitary sewer connection is shown on Sheet C-500.

3.9 Erosion and Sediment Control

Approximately 3.87 acres of the 4.19-acre Site (92%) is proposed to be disturbed as part of this project. Erosion and sediment (E&S) controls will be designed in accordance with the minimum standards and specifications found in the Virginia Erosion and Sediment Control Handbook (VESCH) and the City of Alexandria Erosion and Sediment Control Ordinance. The intent of the Erosion and Sediment control plan is to minimize the extent of the cleared area, install perimeter controls, reduce runoff velocities, and install measures to stabilize disturbed areas. Perimeter controls may include construction entrance and wash rack, sediment traps and basins, diversion dikes, inlet protection, silt fence, and super silt fence. These devices will be installed after the preconstruction meeting and prior to any major earthwork activities. Other erosion and sediment control devices may be installed within the Site to achieve the required reductions in runoff and prevent off-site transport of sediment during and after construction. The limits of clearing based on all anticipated improvements, including buildings, drives and utilities is shown on sheet C-300.

3.10 Anticipated Permits

- City of Alexandria Building & Site Permits
- VSMP Permit

3.11 Anticipated Construction Schedule

Final construction schedule and phasing to be determined pending final permit approval. Estimated total construction duration of 30 months, including:

- Abatement and Demolition: 2 months
- Utilities: 4 months total (not continuous)
- Retaining Walls/Soil Stability/SOE/Excavation: 6 months
- Vertical Construction: 12 months
- Finishes thru Turnover: 12 months

4.0 Discussion

4.1 **RPA Encroachment**

The proposed RPA disturbance will be approximately 0.36 acres and will consist of earthwork, the construction of the proposed building, sidewalks, necessary utility and storm tie ins, tree removal and tree planting, and turf installation. The proposed RPA disturbance will be in an untreated pervious area that is unusable, previously encroached and disturbed. The post construction encroachment will be 0.12 acres of impervious area (building and sidewalks) and 0.06 acres of pervious (grass). The building will be constructed of material consistent with traditional high-density apartment buildings including brick, concrete block, steel, wood and various energy efficient materials. The sidewalk will be constructed of permeable pavement. The entire encroached area will be treated through stormwater management BMPs and pedestrian circulation will be improved. The pervious area will be stabilized with non-invasive grasses native to the mid-Atlantic region. The existing forested area within the RPA will be preserved and supplemented with an additional 0.03 acres of tree planting. Trees planted within the RPA will consist of locally native, non-invasive trees and will be planted to mimic the existing condition within the RPA.

WATER QUALITY IMPACT ASSESSMENT UDR NVII

The land cover area change within the RPA is +0.11 acres for impervious area, -0.14 acres for pervious area and +0.03 acres for forested area. The land cover percent change for the proposed development within the RPA and for the entire site is summarized in Table 8.

Table 8 Land Cover Change											
Land Cover Analysis				RPA Land Cover Analysis							
Cover Type	Ex.	Prop.	Area Change	% Change	Ex.	Prop.	Area Change	% Change			
Impervious	0.99	2.23	1.24	125%	0.01	0.12	0.11	1100%			
Pervious	1.73	1.06	-0.67	-39%	0.22	0.08	-0.14	-64%			
Forested	1.47	0.9	-0.57	-39%	0.47	0.5	0.03	6%			

The proposed encroachment and post construction land cover is shown on sheet C-300.

4.2 Water Quality

The stormwater runoff generated on the existing Site flows uncontrolled and conveys an estimated 1.58 pounds of phosphorus into the adjacent unnamed tributary of Lucky Run. Beauregard Street, which is located directly adjacent to the unnamed tributary of Lucky Run, in the RPA, currently flows uncontrolled into the stream conveying an unquantifiable pollutant load into the stream.

The proposed redevelopment will generate an estimated 2.75 pounds of phosphorus. Seven (7) BMPs have been implemented into the proposed redevelopment and will provide 2.81 pounds of phosphorus load reduction, exceeding the requirement minimum load reduction by 0.06 pounds. See the Table 4 above for a summary of the BMPs implemented on the Site. All impervious areas onsite will be routed through a water quality BMP for quality control and peak flow attenuation before being discharged downstream. The load generated from the Site will be completely mitigated before leaving the site.

The RPA will also be replanted with native, non-invasive trees and grasses that will also provide an unquantifiable runoff reduction through evapotranspiration, and filtering for all stormwater runoff generated in this area.

No direct impacts to wetlands or waters of the U.S. are proposed as part of this project. As a result, no mitigation for WOUS should be required. Onsite stormwater will be collected into BMPs through storm drainage conveyances. The stormwater BMPs will attenuate the 10-year peak flow and discharge predevelopment flow into the adjacent wetlands, streams, lakes rivers or other water bodies after it has been treated in accordance with local, state and federal regulations. Predevelopment flow will provide adequate supply of water to the downstream wetlands, streams, lakes, rivers or other water bodies. As a result, no disruption or reductions in the supply of water to wetlands, streams, lakes, rivers or other water bodies as part of this project. No dredging or disposal of dredging materials is proposed as part of this project limits or outside of the project limits. No drain fields are located on the existing Site. No direct impacts to wetlands or waters of the U.S. are proposed as part of this project. Quantity and quality BMPs have been implemented onsite to treat stormwater discharging to the adjacent stream. As a result, no impacts on adjacent shellfish beds, submerged aquatic vegetation and fish spawning areas are expected as part of this project. The proposed development is not water-dependent.

WATER QUALITY IMPACT ASSESSMENT UDR NVII

The development will not result in unnecessary destruction of plant material. It is assumed that plant material within the limits of disturbance will be removed.

4.3 Additional Water Quality Measures

As documented above, the proposed redevelopment meets the minimum requirements for water quality. As additional mitigation to offset the proposed encroachment into the RPA, the Owner will construct or contribute resources to construct additional water quality measures to demonstrate their commitment to enhance water quality in the City of Alexandria. Those measures include:

- Reforestation (on/off site)
- Streambank Stabilization/Restoration (off site)
- Removal of Slip Lanes (Land Use Change off site)
- Pet Waste Pilot Program

Preliminary POC reduction credit calculations indicate that implementing these measures will provide significant water quality credit that can be applied to the City's TMDL. The Pet Waste Pilot Program can be used to help achieve Minimum Control Measure #1 of the City's TMDL Program Plan. The final program and design is pending City approval.

5.0 Conclusion

The redevelopment to the Newport Village Phase II community will provide a high density, multi family apartments that will be walkable, provide a boost to the local economy through taxes and residents spending locally, and will be environmentally friendly through BMP implementation, POC load reduction, invasive species management, and the implementation of native, non-invasive vegetation throughout the Site, including reforestation of the RPA. The entire RPA where the encroachment is proposed has been disturbed through the development of the existing Site and the construction of Beauregard Street and its sidewalks. The proposed encroachment plans to utilize the previously disturbed area on Site and within the RPA to better the on-Site conditions with a new structure, site stabilization, implementation of water quality BMPs, POC load reduction, and the implementation of native, non-invasive vegetation. These improvements will improve water quality on a site where no previous water quality BMPs were implemented. Furthermore, no WOUS are on the Site, so no impacts to WOUS, SAV, fish, shellfish or any other aquatic species are expected.

The proposed redevelopment has demonstrated that it can fully meet the minimum VSMP requirements. The Owner has committed to going above and beyond to effectively provide additional water quality measures to benefit the City's Chesapeake Bay TMDL POC reduction credits.

This WQIA is based on preliminary information from the Concept Phase 2 Plan Set. This signature and seal certifies that the information contained herein is accurate only to the degree that the conceptual set of plans is accurate at this stage in design and is not based on construction level or final documents.

Signed: John L. Kauppila, P.E.

Dated: November 18, 2019

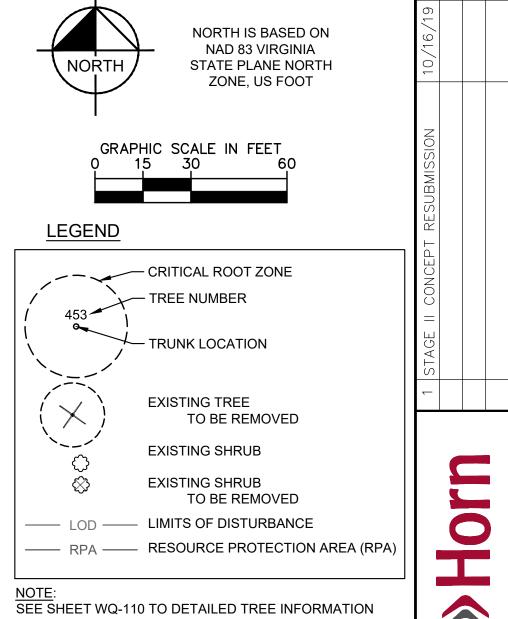


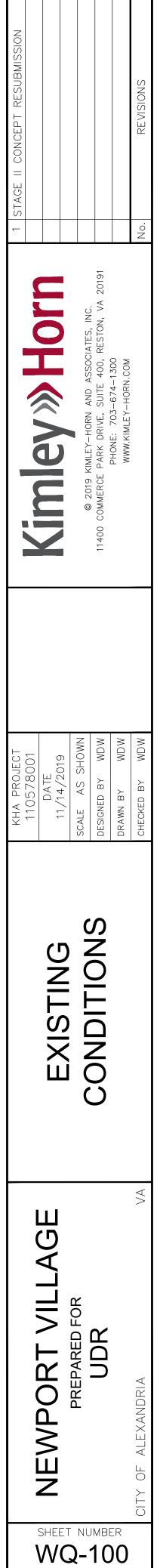
Signed: Karl Mertig - Virginia Certified Professional Wetland Delineator #000089.

Dated: November 18, 2019

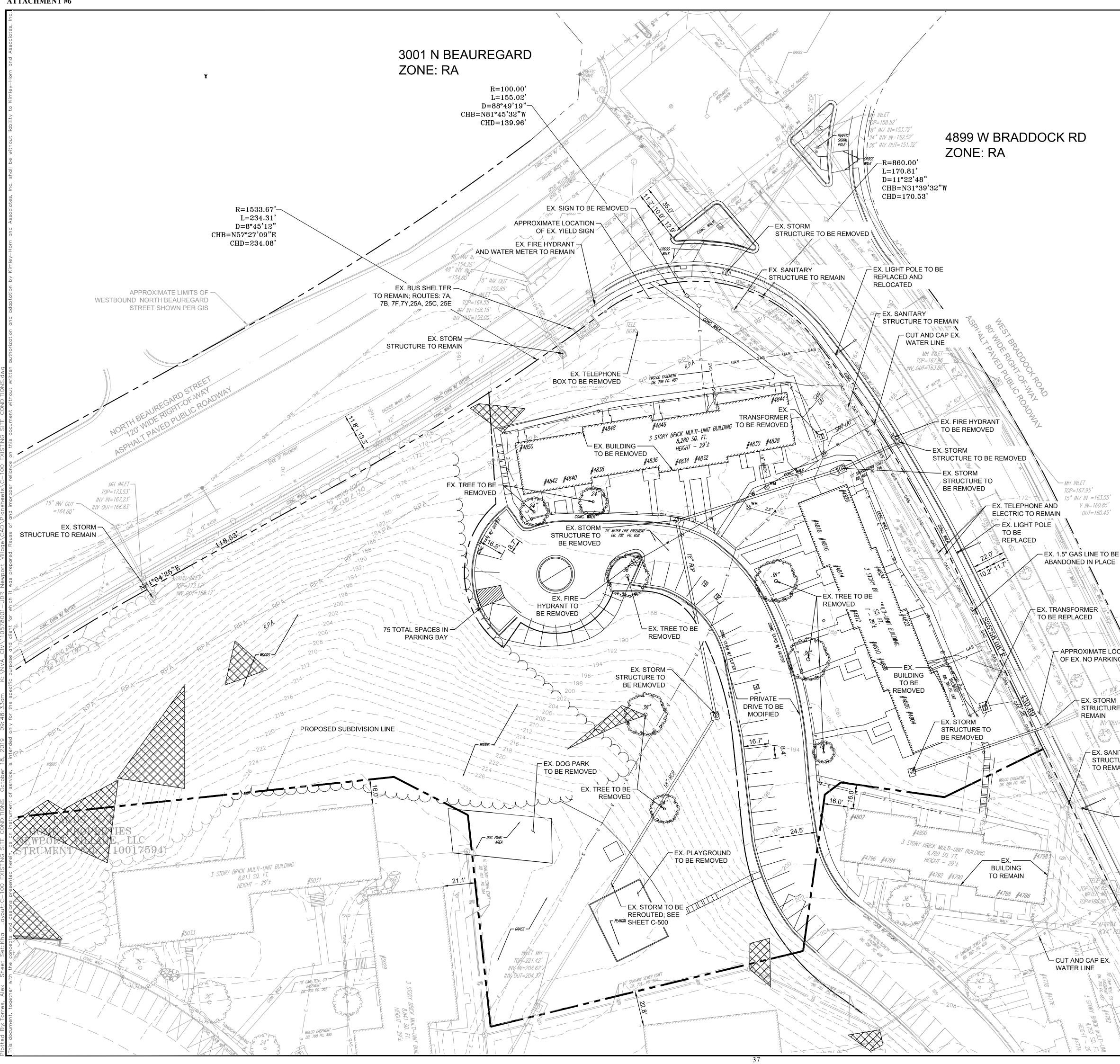
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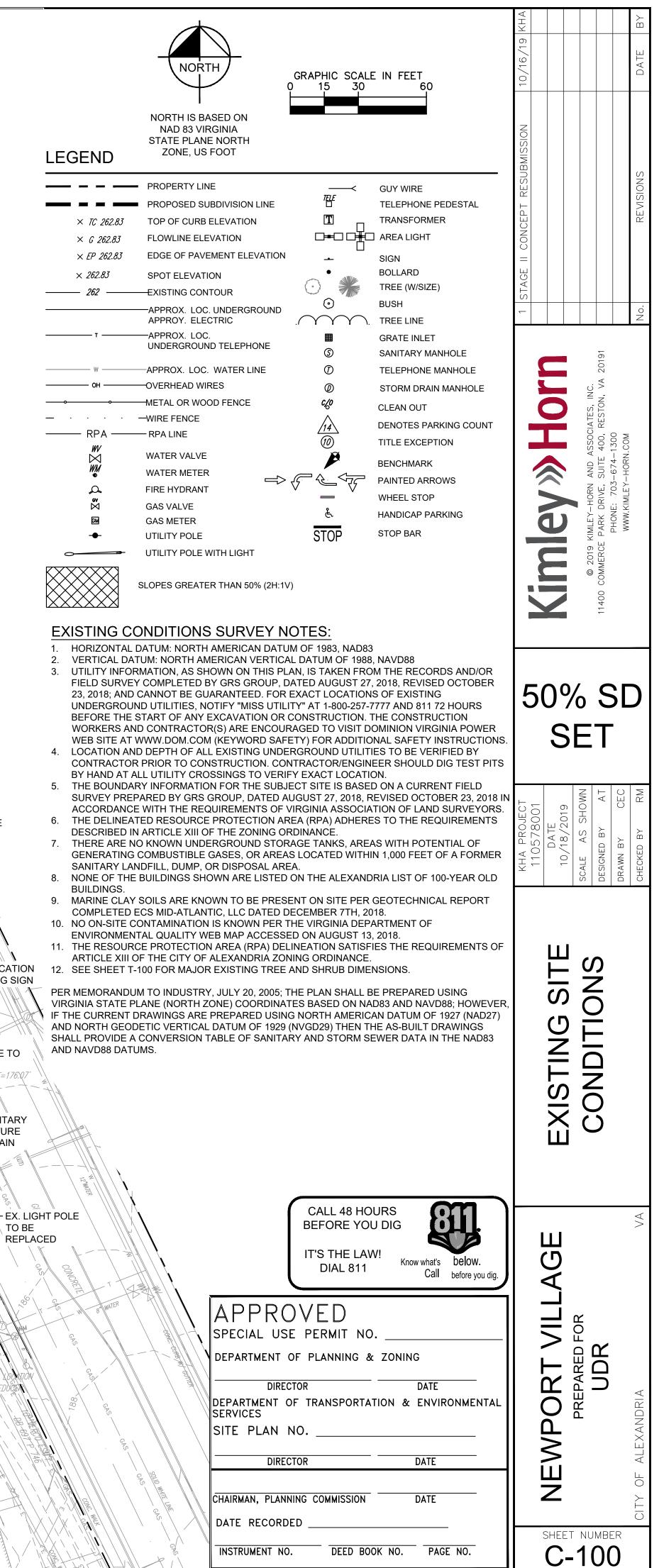












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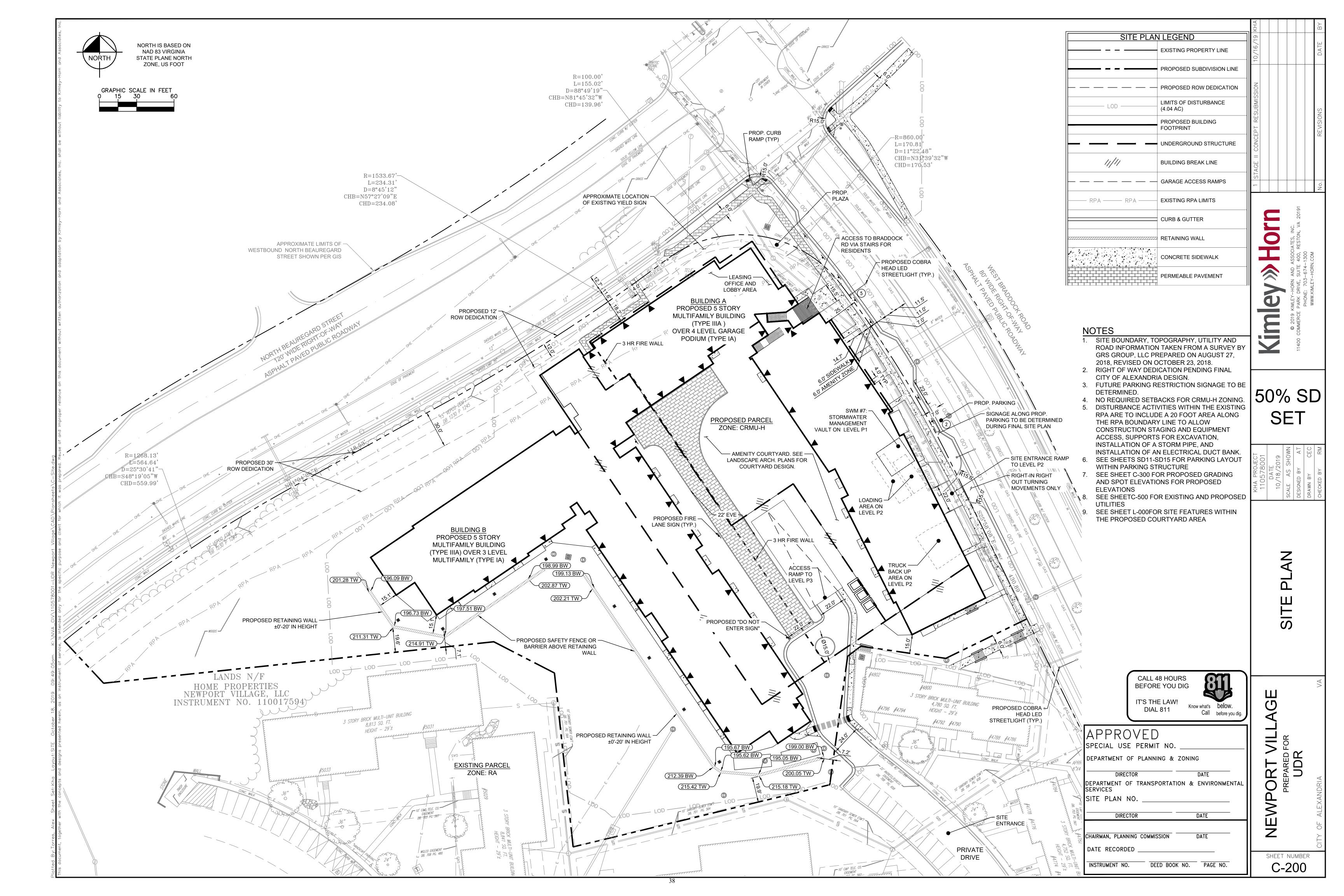
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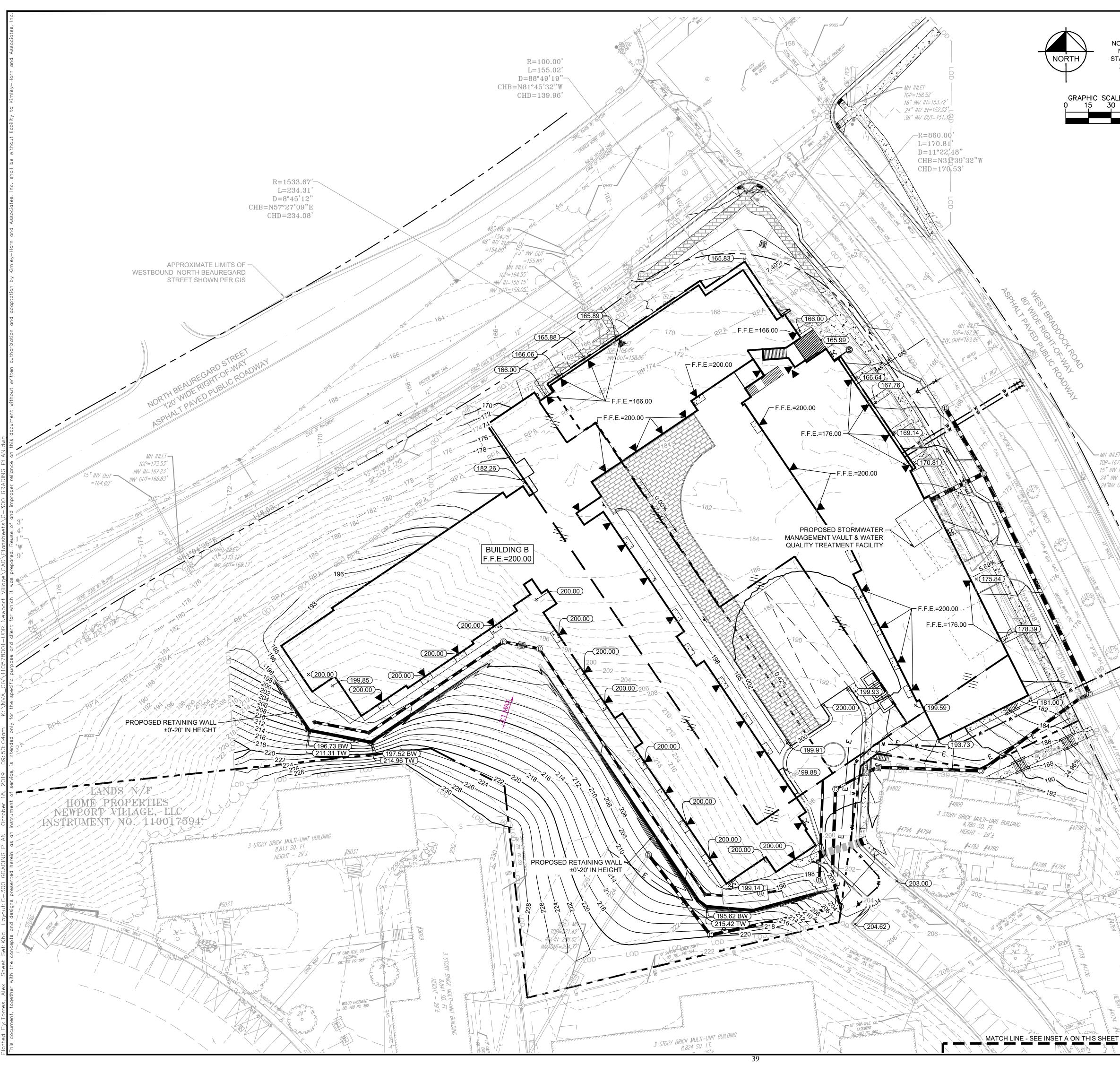
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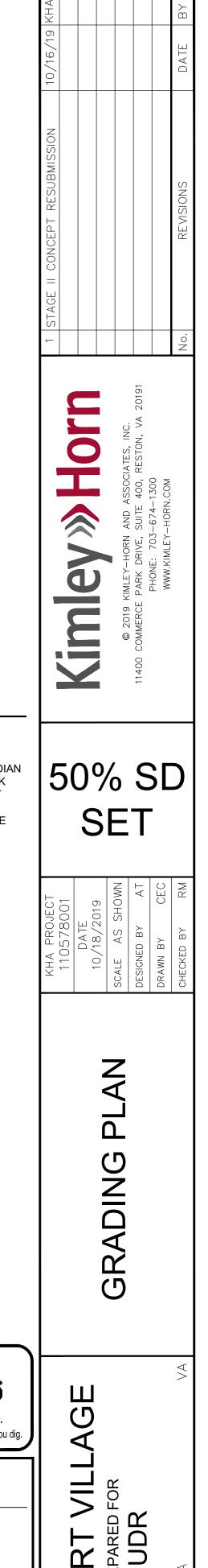
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SHEET NUMBER

C-300

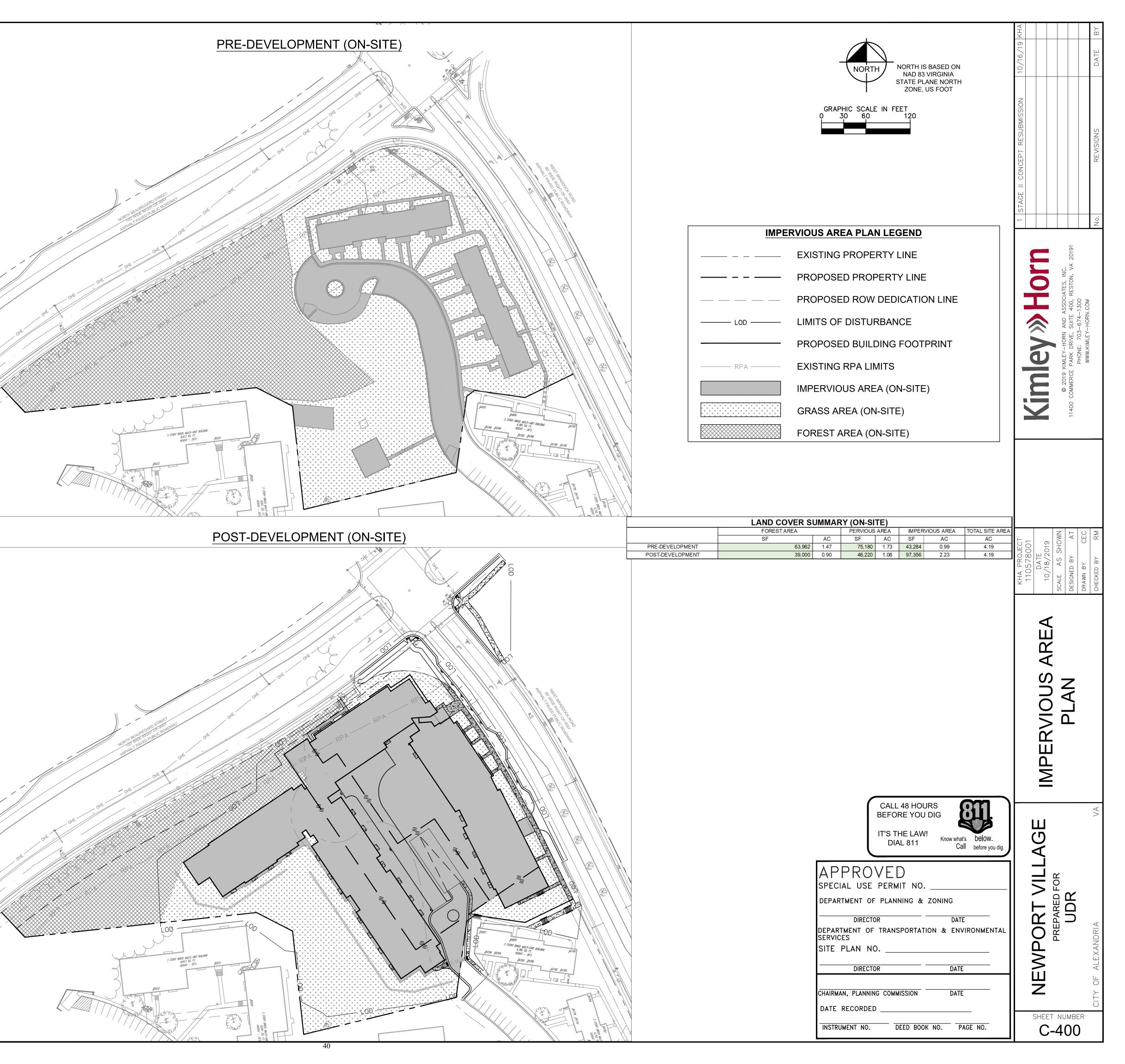
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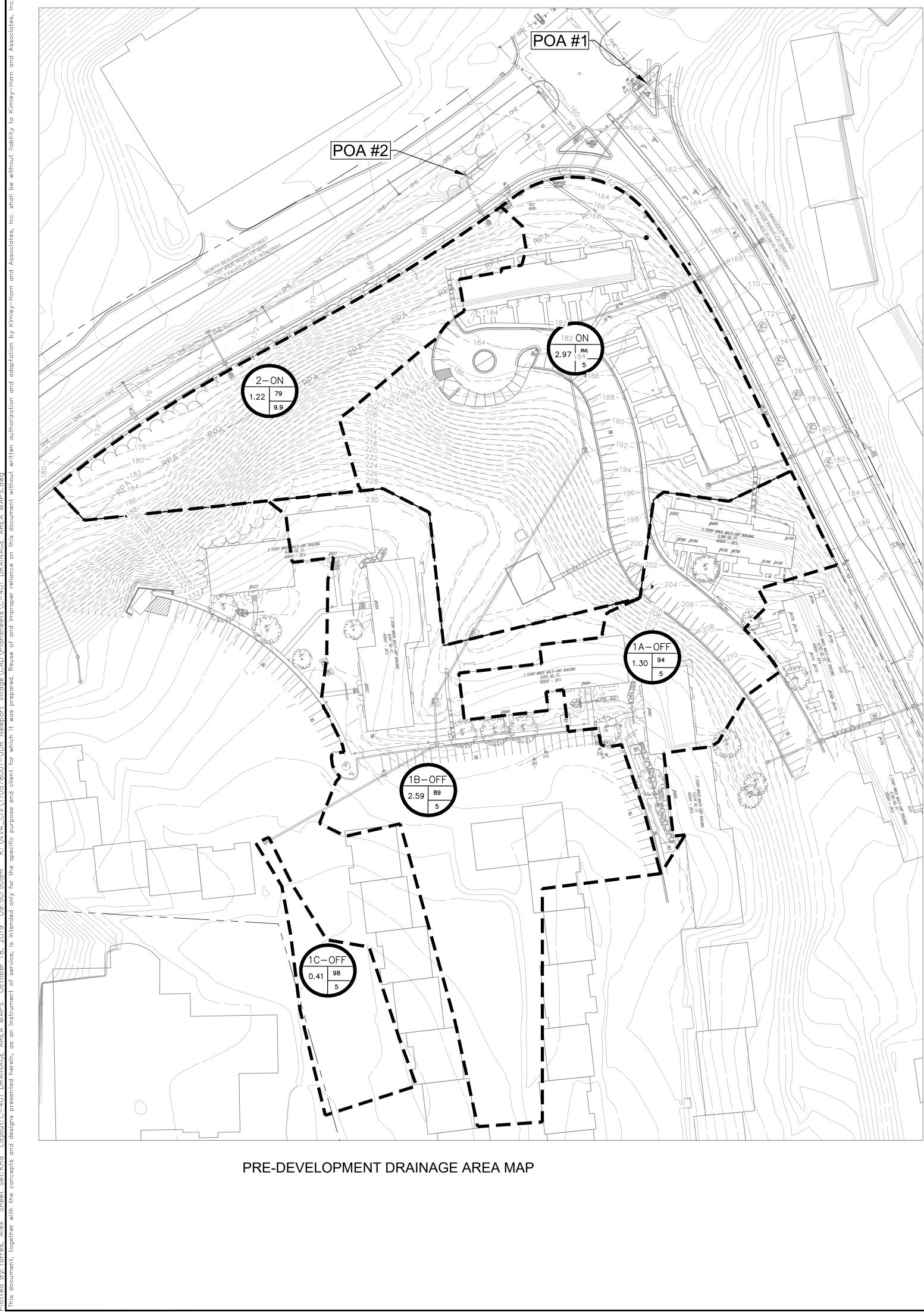
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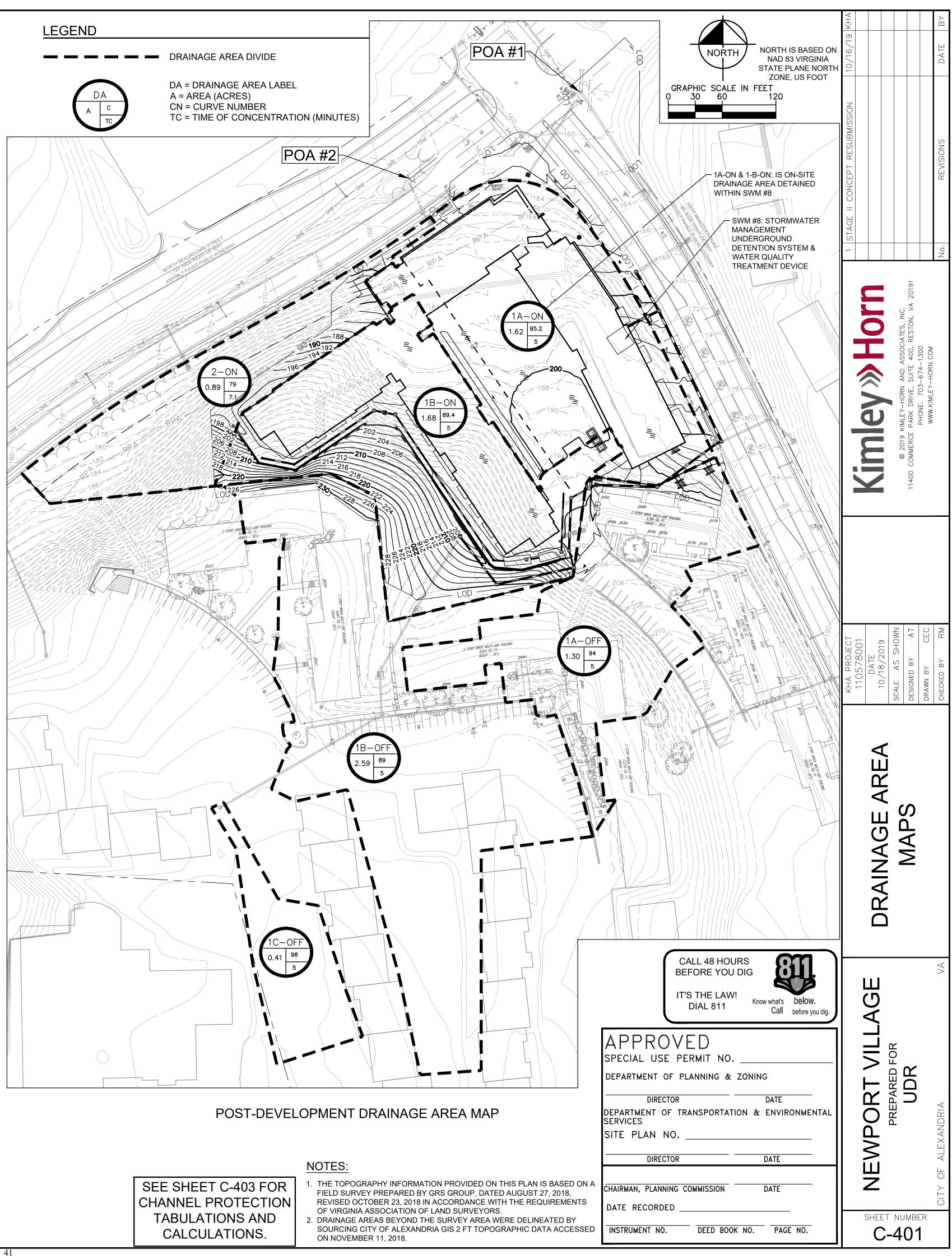
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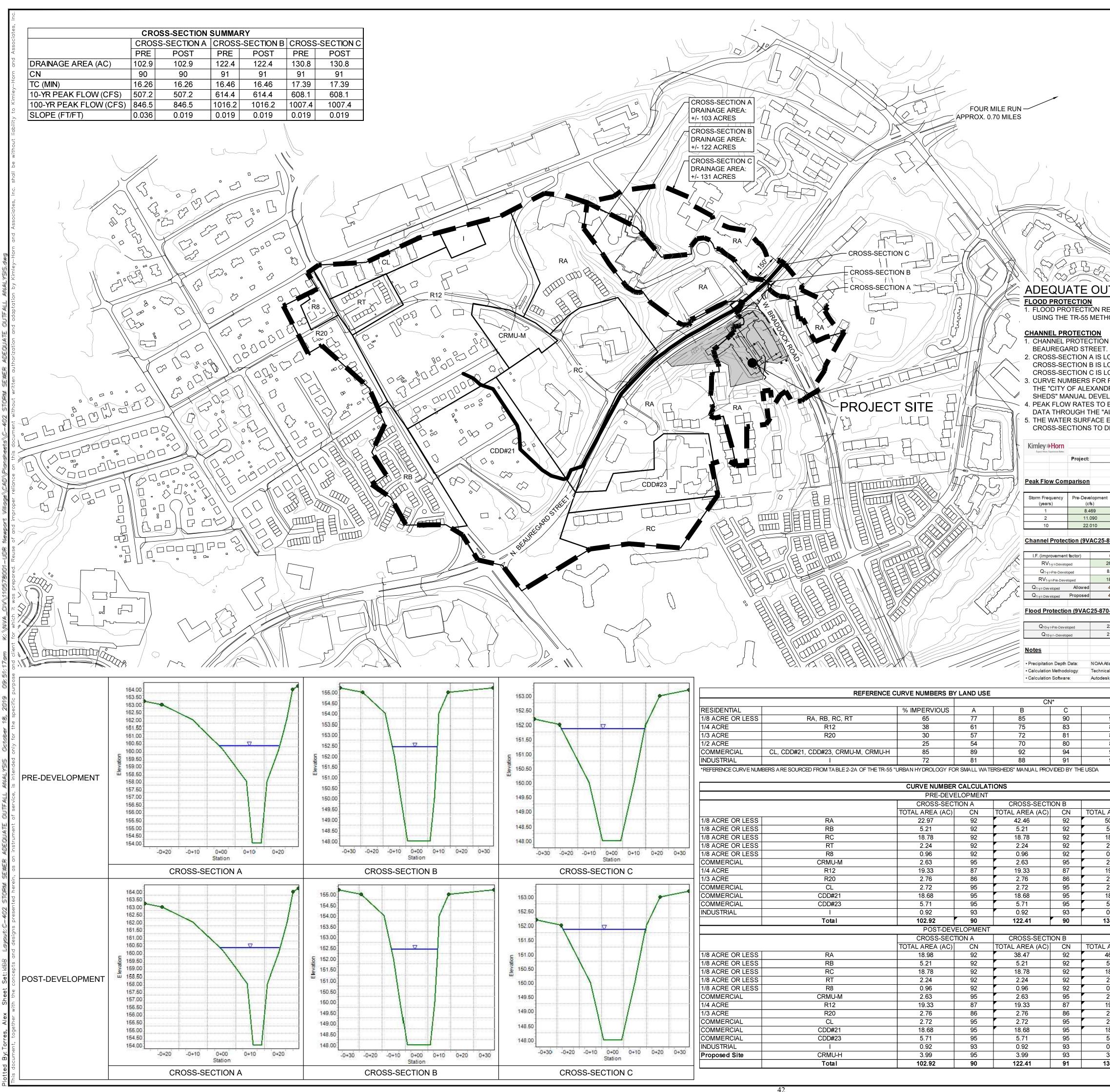
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SEE SHEET C-403 FOR CHANNEL PROTECTION TABULATIONS AND CALCULATIONS.	1. The Fiei Rev OF 2. DRA SOU



	REFERENCE C	URVE NUMBERS B	Y LAND USE			
				10	N*	
RESIDENTIAL		% IMPERVIOUS	A	В	C	
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1/4 ACRE	R12	38	61	75	83	
1/3 ACRE	R20	30	57	72	81	
1/2 ACRE		25	54	70	80	
COMMERCIAL	CL, CDD#21, CDD#23, CRMU-M, CRMU-H	85	89	92	94	
INDUSTRIAL		72	81	88	91	

		CURVE NUMBER				
		PRE-DEVE	ELOPMENT			
		CROSS-SECT	TION A	CROSS-SECTIO	DN B	
		TOTAL AREA (AC)	CN	TOTAL AREA (AC)	CN	TOTAL AR
1/8 ACRE OR LESS	RA	22.97	92	42.46	92	50.8
1/8 ACRE OR LESS	RB	5.21	92	5.21	92	5.2
1/8 ACRE OR LESS	RC	18.78	92	18.78	92	18.7
1/8 ACRE OR LESS	RT	2.24	92	2.24	92	2.24
1/8 ACRE OR LESS	R8	0.96	92	0.96	92	0.90
COMMERCIAL	CRMU-M	2.63	95	2.63	95	2.63
1/4 ACRE	R12	19.33	87	19.33	87	19.3
1/3 ACRE	R20	2.76	86	2.76	86	2.76
COMMERCIAL	CL	2.72	95	2.72	95	2.72
COMMERCIAL	CDD#21	18.68	95	18.68	95	18.6
COMMERCIAL	CDD#23	5.71	95	5.71	95	5.7
INDUSTRIAL		0.92	93	0.92	93	0.92
	Total	102.92	90	122.41	90	130.7
		POST-DEV	ELOPMEN	Т		
		CROSS-SECT	TION A	CROSS-SECTIO)N B	
		TOTAL AREA (AC)	CN	TOTAL AREA (AC)	CN	TOTAL AR
1/8 ACRE OR LESS	RA	18.98	92	38.47	92	46.8
1/8 ACRE OR LESS	RB	5.21	92	5.21	92	5.2
1/8 ACRE OR LESS	RC	18.78	92	18.78	92	18.7
1/8 ACRE OR LESS	RT	2.24	92	2.24	92	2.24
1/8 ACRE OR LESS	R8	0.96	92	0.96	92	0.96
COMMERCIAL	CRMU-M	2.63	95	2.63	95	2.63
1/4 ACRE	R12	19.33	87	19.33	87	19.3
1/3 ACRE	R20	2.76	86	2.76	86	2.76
COMMERCIAL	CL	2.72	95	2.72	95	2.72
COMMERCIAL	CDD#21	18.68	95	18.68	95	18.6
COMMERCIAL	CDD#23	5.71	95	5.71	95	5.7
INDUSTRIAL	I	0.92	93	0.92	93	0.92
Proposed Site	CRMU-H	3.99	95	3.99	93	3.99
	Total	102.92	90	122.41	91	130.7

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4.73 cfs 4.34 cfs 0-66 C) 22.01 cfs 21.16 cfs 21.16 cfs tlas 14 al Release 55 (TR-55) k Hydraflow Hydrographs D 92 87 86 85		RV1-yr-Developed = the volume of runoff from the site in the pre-developed condition Q1-yr-Pre-Developed = the volume of runoff from the site in pre-developed condition Q1-yr-Forest = the peak flow rate of runoff from the site in a forested condition Q1-yr-Forest = the volume of runoff from the site in a forested condition Q1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of	TORM SEWER In 10, 10, 10, 10, 10, 10, 10, 10, 10, 10,
4.73 cfs 4.34 cfs 0-66 C) 22.01 cfs 21.16 cfs 21.16 cfs 1 al Release 55 (TR-55) k Hydraflow Hydrographs D 92 87 86 85 95 93 CROSS-SECTION C		RV 1-yr-Doveloped = the volume of runoff from the site in the pre-developed condition Q1-yr-Pre-Developed = the volume of runoff from the site in pre-developed condition Q1-yr-Pre-Developed = the volume of runoff from the site in a forested condition Q1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition RV1-yr-Forest = the volume of runoff from the site in a forested condition SEE SHEET C-4001 FOR RV1-yr-Forest = the volume of runoff from the site in a forested condition Exect the volume of runoff from th	SEWER EOUTFALL YSIS CHECKE
4.73 cfs 4.34 cfs 4.34 cfs 0-66 C)	CN 92 92	<form></form>	STORM SEWER ADEQUATE OUTFALL ANALYSIS PRAW
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4.73 cfs 4.34 cfs 4.34 cfs 0-66 C)	CN 92 92 92 92 92 92 95 95 95 95 95 95 95 95 93 91	<form><form></form></form>	ADEQUATE OUTFALL SCALE ANALYSIS DRAW PRANCE OUTFALL SCALE ANALYSIS DRAW
4.73 cfs 4.34 cfs 4.34 cfs 0-66 C)	CN 92 92 92 92 92 92 95 95 95 95 95 95 95 95 93 91	<form><form></form></form>	RHA RHA RT VILLAGE STORM SEWER PARED FOR 10, PARED FOR 10, PARED FOR ADEQUATE OUTFALL SCALE BALYSIS
4.73 cfs 4.34 cfs 0-66 C)	CN 92 92 92 92 92 92 95 95 95 95 95 95 95 95 93 91	<form></form>	ORT VILLAGE PREPARED FOR UDR DRAM ADEQUATE OUTFALL ADEQUATE OUTFALL SCALE DESIONE DRAM DRAM
4.73 cfs 4.34 cfs 0-66 C)	CN 92 92 92 92 92 92 92 95 87 86 95 95 95 95 95 95 93 99 91 CN 92 92 92 92 92 92 92	<form></form>	ORT VILLAGE PREPARED FOR UDR DRAM ADEQUATE OUTFALL ADEQUATE OUTFALL SCALE DESIONE DRAM DRAM
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4.73 cfs 4.34 cfs 24.34 cfs 21.16 cfs 22.01 cfs 21.16 cfs 31.14 al Release 55 (TR-55) ik Hydraflow Hydrographs 92 87 86 85 95 93 93 0.92 87 86 85 95 93 0.96 2.24 0.96 2.72 0.96 2.74 0.92 93 0.96 2.72 0.96 9.33 0.96 9.33 <td< td=""><td>CN 92 92 92 92 92 92 92 92 92 92 95 87 86 95 95 95 93 95 93 91 91 CN 92 92 92 92 92 92 92 92 92 92 92 92 92</td><td><form></form></td><td>EWPORT VILLAGE PREPARED FOR PREPARED FOR PREPARED FOR DEC ALEXANDRIA ADEQUATE OUTFALL SCALE CALE DESIGNE DESIG</td></td<>	CN 92 92 92 92 92 92 92 92 92 92 95 87 86 95 95 95 93 95 93 91 91 CN 92 92 92 92 92 92 92 92 92 92 92 92 92	<form></form>	EWPORT VILLAGE PREPARED FOR PREPARED FOR PREPARED FOR DEC ALEXANDRIA ADEQUATE OUTFALL SCALE CALE DESIGNE DESIG

. Hydrograpi type (origin)	h Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
SCS Runof	f 6.974	2	716	14,096				PRE-1-ON
SCS Runof	f 1.725	2	722	4,549				PRE-2-ON
Combine	8.469	2	718	18,646	1, 2			PRE-SITE
SCS Runof	f 6.007	1	717	13,201				POST-1A-ON
SCS Runof	f 5.065	1	717	10,455				POST-1B-ON
SCS Runof	f 1.450	1	719	3,068				POST-2-ON (UNDETAINED)
SCS Runof	f 220.95	2	722	623,126				PRE-CROSS-SECTION A
SCS Runof	f 274.22	2	722	776,955				PRE-CROSS-SECTION B
SCS Runof	f 270.39	2	724	851,430				PRE-CROSS-SECTION C
SCS Runof	f 220.95	2	722	623,126				POST-CROSS-SECTION A
SCS Runof	f 274.22	2	722	776,955				POST-CROSS-SECTION B
SCS Runof	f 270.39	2	724	851,430				POST-CROSS-SECTION C
Combine	11.07	1	717	23,657	4, 5,			POST-1 (DETAINED)
Reservoir	3.166	1	725	23,652	13	174.70	7,520	SWM #8
Combine	4.335	1	720	26,720	6, 14			POST-SITE

10-YEAR PEAK FLOW SUMMARY

Hydrograph Summary Report

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	17.59	2	716	36,728				PRE-1-ON
2	SCS Runoff	5.298	2	720	13,755				PRE-2-ON
3	Combine	22.01	2	718	50,483	1, 2			PRE-SITE
4	SCS Runoff	12.20	1	717	28,141				POST-1A-ON
5	SCS Runoff	11.65	1	717	25,193				POST-1B-ON
6	SCS Runoff	4.396	1	718	9,370				POST-2-ON (UNDETAINED)
7	SCS Runoff	507.24	2	722	1,482,325				PRE-CROSS-SECTION A
В	SCS Runoff	614.37	2	722	1,809,404				PRE-CROSS-SECTION B
9	SCS Runoff	608.13	2	724	1,982,844				PRE-CROSS-SECTION C
10	SCS Runoff	507.24	2	722	1,482,325				POST-CROSS-SECTION A
11	SCS Runoff	614.37	2	722	1,809,404				POST-CROSS-SECTION B
12	SCS Runoff	608.13	2	724	1,982,844				POST-CROSS-SECTION C
13	Combine	23.84	1	717	53,333	4, 5,			POST-1 (DETAINED)
14	Reservoir	17.37	1	721	53,328	13	177.51	12,007	SWM #8
15	Combine	21.16	1	720	62,698	6, 14			POST-SITE
UD	R Newport V	illage.gpw	,		Return P	veriod: 10	Year	Wednesda	ay, 10 / 16 / 2019

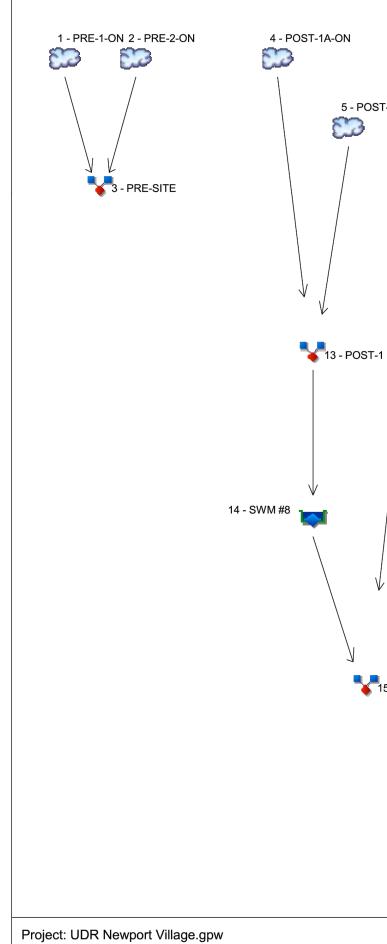
2-YEAR PEAK FLOW SUMMARY

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	9.053	2	716	18,390				PRE-1-ON
2	SCS Runoff	2.378	2	722	6,223				PRE-2-ON
3	Combine	11.09	2	718	24,612	1, 2			PRE-SITE
4	SCS Runoff	7.257	1	717	16,165				POST-1A-ON
5	SCS Runoff	6.380	1	717	13,308				POST-1B-ON
6	SCS Runoff	1.997	1	719	4,211				POST-2-ON (UNDETAINED)
7	SCS Runoff	278.12	2	722	789,933				PRE-CROSS-SECTION A
8	SCS Runoff	342.43	2	722	978,368				PRE-CROSS-SECTION B
9	SCS Runoff	338.06	2	724	1,072,150				PRE-CROSS-SECTION C
10	SCS Runoff	278.12	2	722	789,933				POST-CROSS-SECTION A
11	SCS Runoff	342.43	2	722	978,368				POST-CROSS-SECTION B
12	SCS Runoff	338.06	2	724	1,072,150				POST-CROSS-SECTION C
13	Combine	13.64	1	717	29,472	4, 5,			POST-1 (DETAINED)
14	Reservoir	8.194	1	722	29,467	13	175.32	8,505	SWM #8
15	Combine	9.747	1	722	33,678	6, 14			POST-SITE

ROUTING MAP

2

Watershed Model Schematic



3

100-YEAR PEAK FLOW SUMMARY

Hydrograph Summary Report Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020 Hyd.HydrographPeakTimeTime toHyd.No.typeflowintervalPeakvolume(origin)(cfs)(min)(min)(cuft) Inflow hyd(s) Hydrograph Description Total Maximum elevation strge used (ft) (cuft) 1 SCS Runoff 30.40 2 716 65,713 PRE-1-ON -----_____ _____ 9.920 2 SCS Runoff 2 720 26,222 PRE-2-ON _____ _____ _____ 3 Combine 38.61 2 718 91,936 1, 2 -----PRE-SITE -----19.52 717 46,238 4 SCS Runoff 1 -----POST-1A-ON _____ -----5 SCS Runoff 19.44 1 717 43,588 POST-1B-ON -----_____ -----718 17,937 POST-2-ON (UNDETAINED) 6 SCS Runoff 8.192 1 _____ _____ -----722 2,550,726 ------PRE-CROSS-SECTION A 7 SCS Runoff 846.48 2 -----_____ PRE-CROSS-SECTION B 8 SCS Runoff 1016.17 2 722 3,085,522 ---------------724 3,381,286 -----9 SCS Runoff 1007.40 2 PRE-CROSS-SECTION C ----------POST-CROSS-SECTION A 10 SCS Runoff 846.48 2 722 2,550,726 ----------_____ 11 SCS Runoff 1016.17 2 722 3,085,522 ------POST-CROSS-SECTION B ----------12 SCS Runoff 1007.40 2 724 3,381,286 POST-CROSS-SECTION C _____ ----------717 89,826 4, 5, POST-1 (DETAINED) 13 Combine 38.96 1 -----_____ 1 718 89,821 13 177.93 12,684 14 Reservoir 38.54 SWM #8 718 107,758 6, 14 46.73 1 POST-SITE 15 Combine _____ -----Return Period: 100 Year Wednesday, 10 / 16 / 2019 UDR Newport Village.gpw

4

43

1 Now Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020	10/16/19 KHA
7 - PRE-CROSS-SECTION A	NOIS
8 - PRE-CROSS-SECTION B	EPT RESUBMISSION
6 - POST-2-ON (UNDETAINED) 9 - PRE-CROSS-SECTION C	STAGE IL CONC
1 (DETAINED) 10 - POST-CROSS-SECTION A	
11 - POST-CROSS-SECTION B	OTD TES, INC. ESTON, VA 2019
12 - POST-CROSS-SECTION C	Cimpley Morn © 2019 KIMLEY-HORN AND ASSOCIATES, INC. 1400 COMMERCE PARK DRIVE, SUITE 400, RESTON, VA 20191 PHONE: 703-674-1300 WWW.KIMLEY-HORN.COM
15 - POST-SITE	© 2019 KIMLEY-HORN 11400 COMMERCE PARK DRIVE, PHONE: 703, WWW.KIMLEY-
Wednesday, 10 / 16 / 2019 Wednesday, 10 / 16 / 2019 SEE SHEET C-401 FOR DRAINAGE DIVIDES AND C-402 FOR AREA CALCULATIONS.	
UALOOLAHONO.	KHA PROJECT 110578001 DATE 10/18/2019 CALE AS SHOWN SIGNED BY AT ESIGNED BY AT RAWN BY CEC HECKED BY RM
	KHA PI 11057 10/18 Scale A Designed t Drawn BY CHECKED E
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CALL 48 HOURS BEFORE YOU DIG IT'S THE LAW! DIAL 811 Know what's below. Call before you dig.	В М
APPROVED SPECIAL USE PERMIT NO.	
DEPARTMENT OF PLANNING & ZONING DIRECTOR DATE	PREPARED FOR UDR
DEPARTMENT OF TRANSPORTATION & ENVIRONMENTAL SERVICES SITE PLAN NO	
DIRECTOR DATE DATE CHAIRMAN, PLANNING COMMISSION DATE	NEWP(
DATE RECORDED	SHEET NUMBER

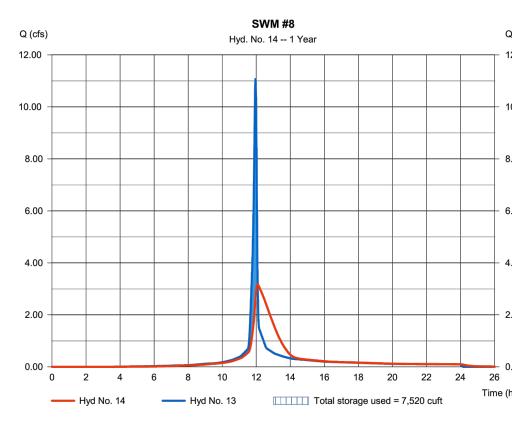
Туре	PRE-1 Desc.	Length (ft.)	Elev Up (ft.)	Elev. Dn (ft.)	Slope (ft./ft.)	P2 (in.)	Cover Type	n value	Diameter (ft.)	Width (ft.)	S. Slopes H:V	Depth (ft.)	Velocity (fps)	Tc (min)	Area (sq. ft.)	Rh	Q (cfs)
1	Overland Flow (TR-55)	100	255.00	253.00	0.020	3.2	G	0.150						9.80			
2	Shallow Conc. 1	980	253.00	214.00	0.040		U						3.22	5.07			
	Shallow Conc. 2	170			0.000		P	0.040	0.5				0.00	0.00			0.15
3	Pipe 1 Pipe 2	473 223	214.00 186	186 180	0.059			0.013	3.5				25.50	0.31	9.6	0.9	245
3	Channel 1	813	180.00	154.00	0.0320			0.045		5	2.5	8	15.32	0.88	200.0	4.2	3063
3	Pipe 3	0	154	148	0.000			0.013	4				0.00	0.00	12.6	1.0	0
	Channel 3	0	148.00	147.75	0.000			0.050		6	1.67	6	0.00	0.00	96.0	3.3	0
	Total Length	2589	<u> </u>		0.0367								Total Tc (mins) Total Travel Time, Tc (hrs)	16.26 0.271			
													Lag Time (hrs) Lag Time (min)	0.163 9.758			
													Avg. Velocity (ft/s)	2.7			
scriptio	n		low Manning'		Code C	n value 0.011		Description	n	Table 2	. Typical Pip	e and Chann	nel Manning's n values	n value			
re Soil	faces: concrete, asphalt, p	acked soll et	<u> </u>		B S	0.060		Concrete Corrugated Concrete ch	Metal Pipe (CN	/IP)				0.013 0.024 0.018			
ort grass	3				G D	0.150		Grass chan		ined				0.035			
	s/Bermuda Grass avy Cover				W H	0.400 0.800		Rip-rap char Channels w	nnel ith significant	weeds and f	ow obs.			0.040 0.075			
paved ved					U P	16.1345*(slo 20.3282*(slo											
	Cross-Section B - Lag	Time Calcı	lations]
Seg.	PRE-1	Length	⊟ev Up	⊟ev. Dn	Slope	P2	Cover	n value	Pipe Diameter	Channel Width	Channel S. Slopes	Channel Depth	Velocity	Тс	Area	Rh	Q
Туре	Desc.	(ft.)	(ft.)	(ft.)	(ft./ft.)	(in.)	Туре	0.450	(ft.)	(ft.)	H:V	(ft.)	(fps)	(min)	(sq. ft.)		(cfs)
1	Overland Flow (TR-55) Shallow Conc. 1	980	255.00 253.00	253.00 214.00	0.020	3.2	G U	0.150					3.22	9.80			
-	Shallow Conc. 2				0.040		P						0.00	0.00			
3	Pipe 1	473	214.00	186	0.059			0.013	3.5				25.50	0.31	9.6	0.9	245
3	Pipe 2	223	186	180	0.027			0.013	4				18.80	0.20	12.6	1.0	236
3	Channel 1	813	180.00	154.00	0.0320			0.045		5	2.5	8	15.32	0.88	200.0	4.2	3063
3	Pipe 3	223	154	148	0.027			0.013	4				18.80	0.20	12.6	1.0	236
	Channel 3	0	148.00	147.75	0.000			0.050		6	1.67	6	0.00	0.00	96.0	3.3	0
	Total Length	2812			0.0359								Total Tc (mins) Total Travel Time, Tc (hrs)	16.46 0.274			
				 									Lag Time (hrs) Lag Time (min) Avg. Velocity (ft/s)	0.165 9.877 2.8			
	Table 1.	Overland Fl	ow Manning'	s n values						Table 2	Typical Pipe	and Chann	el Manning's n values	2.0			
scriptio nooth sur					Code C	n value 0.011		Description Concrete	1		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			n value 0.013			
re Soil Itivated s					B S G	0.060		Concrete ch		IP)				0.024			
ort grass nse gras ht woods					D W	0.150 0.240 0.400		Grass chanr Grass chanr Rip-rap char	nel not maintai	ned				0.035 0.050 0.040			
	avy Cover					0.800 16.1345*(slo			th significant	weeds and fl	ow obs.			0.075			
ved					P	20.3282*(slo	pe)^0.5]									
									Pipe	Channel	Channel	Channel					
	Cross-Section C - Lag				01		0						1 1/21-21		Area		
	Cross-Section C - Lag PRE-1 Desc.	Length	Ulations	Eev. Dn (ft.)	Slope (ft./ft.)	P2 (in.)	Cover Type	n value	Diameter (ft.)	Width (ft.)	S. Slopes H:V	Depth (ft.)	Velocity (fps)	Tc (min)	(sq. ft.)	Rh	Q (cfs)
	PRE-1	Length	Elev Up					n value 0.150					-			Rn	
	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1	Length (ft.)	Elev Up (ft.)	(ft.)	(ft./ft.) 0.020 0.040	(in.)	Type G U						(fps)	(min) 9.80 5.07		Kn	
Type 1 2	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2	Length (ft.) 100 980	Elev Up (ft.) 255.00 253.00	(ft.) 253.00 214.00	(ft./ft.) 0.020 0.040 0.000	(in.)	Type G	0.150	(ft.)				(fps) 3.22 0.00	(min) 9.80 5.07 0.00	(sq. ft.)		(cfs)
Type 1 2 3	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1	Length (ft.) 100 980 473	Elev Up (ft.) 255.00 253.00 214.00	(ft.) 253.00 214.00 186	(ft./ft.) 0.020 0.040 0.000 0.059	(in.)	Type G U	0.150	(ft.) 3.5				(fps) 3.22 0.00 25.50	(min) 9.80 5.07 0.00 0.31	(sq. ft.)	0.9	(cfs) 245
Type 1 2	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2	Length (ft.) 100 980	Elev Up (ft.) 255.00 253.00	(ft.) 253.00 214.00	(ft./ft.) 0.020 0.040 0.000	(in.)	Type G U	0.150	(ft.)				(fps) 3.22 0.00	(min) 9.80 5.07 0.00	(sq. ft.)		(cfs)
Type 1 2 3 3 3	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 2	Length (ft.) 100 980 473 223	Elev Up (ft.) 255.00 253.00 214.00 186	(ft.) 253.00 214.00 186 180	(ft./ft.) 0.020 0.040 0.000 0.059 0.027	(in.)	Type G U	0.150	(ft.) 3.5	(ft.)	H:V	(ft.)	(fps) 3.22 0.00 25.50 18.80	(min) 9.80 5.07 0.00 0.31 0.20	(sq. ft.) 9.6 12.6	0.9	(cfs) 245 236
Type 1 2 3 3 3 3	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 2 Channel 1	Length (ft.) 100 980 473 223 813	Elev Up (ft.) 255.00 253.00 253.00 214.00 186 180.00	(ft.) 253.00 214.00 186 180 154.00	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027	(in.)	Type G U	0.150 0.013 0.013 0.045	(ft.) 3.5 4	(ft.)	H:V	(ft.)	(fps) 3.22 0.00 25.50 18.80 15.32	(min) 9.80 9.80 5.07 0.00 0.00 0.31 0.20 0.20 0.88	(sq. ft.) 9.6 12.6 200.0	0.9 1.0 4.2	(cfs) 245 236 3063
Type 1 2 3 3 3 3 3 3	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3	Length (ft.) 100 980 473 223 813 223	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154	(ft.) 253.00 214.00 186 180 154.00 148	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.0320 0.027	(in.)	Type G U	0.150 0.013 0.013 0.045 0.013	(ft.) 3.5 4	(ft.)	H:V	(ft.)	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins)	(min) 9.80 9.80 5.07 0.00 0.00 0.31 0.31 0.20 0.20 0.88 0.88 0.20 0.20 0.20 0.93 17.39	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 2 3 3 3 3 3 3	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3	Length (ft.) 100 980 473 223 813 223 813 223	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154	(ft.) 253.00 214.00 186 180 154.00 148	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.0320 0.027 0.002	(in.)	Type G U	0.150 0.013 0.013 0.045 0.013	(ft.) 3.5 4	(ft.)	H:V	(ft.)	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs)	(min) 9.80 9.80 5.07 0.00 0.00 0.31 0.31 0.20 0.290 0.290 0.174	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 2 3 3 3 3 3 3	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3	Length (ft.) 100 980 473 223 813 223 813 223	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154	(ft.) 253.00 214.00 186 180 154.00 148	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.0320 0.027 0.002	(in.)	Type G U	0.150 0.013 0.013 0.045 0.013	(ft.) 3.5 4	(ft.)	H:V	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs)	(min) 9.80 9.80 5.07 0.00 0.00 0.31 0.31 0.20 0.20 0.88 0.88 0.20 0.20 0.20 0.93 0.93 17.39 0.290	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 2 3 3 3 3 3 3 scriptio	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3 Total Length Total Length	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.0320 0.027 0.002 0.002 0.002 0.002 0.002 0.002	(in.) 3.2 3.2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	Type G U P	0.150 0.013 0.013 0.045 0.013 0.050 0.050	(ft.) 3.5 4 4	(ft.)	H:V 2.5 1.67	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (min)	(min) 9.80 9.80 0.00 0.00 0.00 0.00 0.00 0.00 0.20 0.174 10.436 2.8	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
3 3 3 3 3 scriptio	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. m	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.0320 0.027 0.002 0.002 0.0342 0.0	(in.) 3.2 	Type G U P	0.150 0.013 0.013 0.045 0.045 0.050 0.050 Description Concrete Corrugated M	(ft.) 3.5 4 4 4	(ft.)	H:V 2.5 1.67	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 0.00 0.00 0.00 0.00 0.31 0.20 0.174 10.436 2.8 0.013 0.024	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. faces: concrete, asphalt, p	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.03	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150	Type G U P	0.150 0.013 0.013 0.045 0.045 0.013 0.050 0.050 Description Concrete Corrugated M Concrete cha Grass chann	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed	(ft.) 5 6 7 7 7 8	H:V 2.5 1.67	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.31 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.018 0.035	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. faces: concrete, asphalt, p	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.0320 0.027 0.002 0.027 0.002 0.0342 0.0355 0.0342 0.03	(in.) 3.2 3.2 n value 0.011 0.060 0.120	Type G U P Image: Second	0.150 0.013 0.013 0.045 0.045 0.013 0.050 0.050 Description Concrete Corrugated M Concrete cha	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed el not maintair nel	(ft.) (ft.) 5 6 7 Table 2. P) ned	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 0.00 0.00 0.00 0.00 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.013 0.024 0.018	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. faces: concrete, asphalt, p	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.002 0.0342 0.002	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400	Type G U P	0.150 0.013 0.013 0.045 0.045 0.013 0.0500 0.0500 0.0500000000	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed el not maintair nel	(ft.) (ft.) 5 6 7 Table 2. P) ned	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.035 0.050 0.040	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. faces: concrete, asphalt, p	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.002 0.0342 0.002	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400 0.800 16.1345*(slop	Type G U P	0.150 0.013 0.013 0.045 0.045 0.013 0.0500 0.0500 0.0500000000	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed el not maintair nel	(ft.) (ft.) 5 6 7 Table 2. P) ned	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.035 0.050 0.040	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. faces: concrete, asphalt, p	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.002 0.0342 0.002	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400 0.800 16.1345*(slop	Type G U P	0.150 0.013 0.013 0.045 0.045 0.013 0.0500 0.0500 0.0500000000	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed el not maintair nel	(ft.) (ft.) 5 6 7 Table 2. P) ned	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.035 0.050 0.040	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Pipe 2 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Table 1. faces: concrete, asphalt, p	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 2962 Overland Fl	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.002 0.0342 0.002	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400 0.800 16.1345*(slop	Type G U P	0.150 0.013 0.013 0.045 0.045 0.013 0.0500 0.0500 0.0500000000	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed el not maintair nel	(ft.) (ft.) 5 6 7 Table 2. P) ned	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.035 0.050 0.040	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 3 5 5 5 5 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Total Length Table 1. m faces: concrete, asphalt, p soils avg. Sises s/Bermuda Grass	Length (ft.) 100 980 473 223 813 223 150 2962 Overland Fl acked soil et	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.002 0.0342 0.002	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400 0.800 16.1345*(slop	Type G U P	0.150 0.013 0.013 0.045 0.045 0.013 0.0500 0.0500 0.0500000000	(ft.) 3.5 4 4 4 Metal Pipe (CM annel el mow ed el not maintair nel	(ft.) (ft.) 5 6 7 Table 2. P) ned	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (hrs) Lag Time (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.31 0.20 0.174 10.436 2.8 0.013 0.024 0.035 0.050 0.040	(sq. ft.) 9.6 12.6 12.6 12.6	0.9 1.0 4.2 1.0	(cfs) 245 236 3063 236
Type 1 1 2 3 3 3 3 3 3 3 3 3 3 3 3 3 5 5 5 5 5 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Channel 1 Pipe 3 Channel 3 Channel 3 Channel 3 Total Length faces: concrete, asphalt, p faces: concrete, asphalt, p soils avg. Sees S/Bermuda Grass	Length (ft.) 100 980 473 223 813 223 150 2962 Overland Fl acked soil et	Elev Up (ft.) 255.00 253.00 214.00 186 180.00 154 148.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.002 0.027 0.002 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.0342 0.002 0.0342 0.002 0.	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400 0.400 0.800 16.1345*(slop 20.3282*(slop	Type G U P	0.150 0.013 0.013 0.045 0.045 0.050	(ft.) 3.5 4 4 4 4 4 4 4 4 4 4 4 4 4	(ft.) 5 6 7 7 8 7 7 8 7 9 7 9	H:V 2.5 1.67 Typical Pipe	(ft.) (ft.) 8 6 Av and Channe	(fps) 3.22 0.00 25.50 18.80 15.32 18.80 2.68 Total Tc (mins) Total Travel Time, Tc (hrs) Lag Time (min) vg. Velocity (ft/s)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.174 10.436 2.8 10.436 2.8 n value 0.013 0.024 0.013 0.024 0.013 0.035 0.050 0.040 0.075	(sq. ft.) (sq. ft.) 9.6 9.6 12.6 96.0 12.6 96.0 12.6	0.9 1.0 4.2 1.0 3.3	(cfs) 245 236 3063 236 258
Type 1 1 2 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 5	PRE-1 Desc. Overland Flow (TR-55) Shallow Conc. 1 Shallow Conc. 2 Pipe 1 Pipe 1 Channel 1 Pipe 3 Channel 3 Channel 3 Total Length Channel 3 Total Length Channel 3 Sises S/Bermuda Grass S/Bermuda Grass	Length (ft.) 100 980 473 223 813 223 813 223 150 2962 Overland Fl acked soil et	Elev Up (ft.) 255.00 253.00 253.00 214.00 186 180.00 154 148.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	(ft.) 253.00 214.00 186 180 154.00 148 147.75 	(ft./ft.) 0.020 0.040 0.000 0.059 0.027 0.027 0.027 0.027 0.002 0.0342 0.0342 0.0342 0.027 0.002 0.0342 0.002 0.0342 0.002	(in.) 3.2 3.2 n value 0.011 0.060 0.120 0.150 0.240 0.400 0.120 0.150 0.240 0.400 0.3282*(slop 20.3282*(slop	Type G U P Image: Second state	0.150 0.013 0.013 0.045 0.045 0.045 0.050	(ft.) 3.5 4 4 4 4 4 4 4 4 4 4 4 4 4	(ft.)	H:V 2.5 1.67 Typical Pipe w obs. Flow W rea (sf) Perin	(ft.) (f	(fps)	(min) 9.80 9.80 5.07 0.00 0.00 0.00 0.00 0.31 0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.20 0.174 10.436 2.8 10.436 2.8 n value 0.013 0.024 0.013 0.024 0.013 0.035 0.050 0.040 0.075	(sq. ft.) (sq. ft.) 9.6 9.6 12.6 96.0 12.6 96.0 12.6	0.9 1.0 4.2 1.0 3.3 ead Specific (f	(cfs) 245 236 3063 236 258

NOTE: FLOWMASTER PROGRAM USED FOR STREAM ANALYSIS

Hydrograph Report

Storage Indication method used.

Hydraflow Hydrographs Extension	on for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2	2020
Hyd. No. 14		
SWM #8		
Hydrograph type Storm frequency Time interval Inflow hyd. No. Reservoir name	= Reservoir = 1 yrs = 1 min = 13 - POST-1 (DETAINED) = SWM #8	Peak discharge Time to peak Hyd. volume Max. Elevation Max. Storage



Hydrograph Report

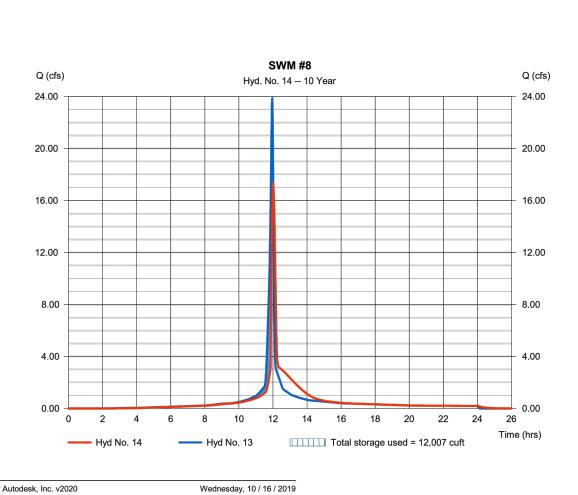
Hydrograph type

Storage Indication method used.

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020 Hyd. No. 14 SWM #8

= Reservoir

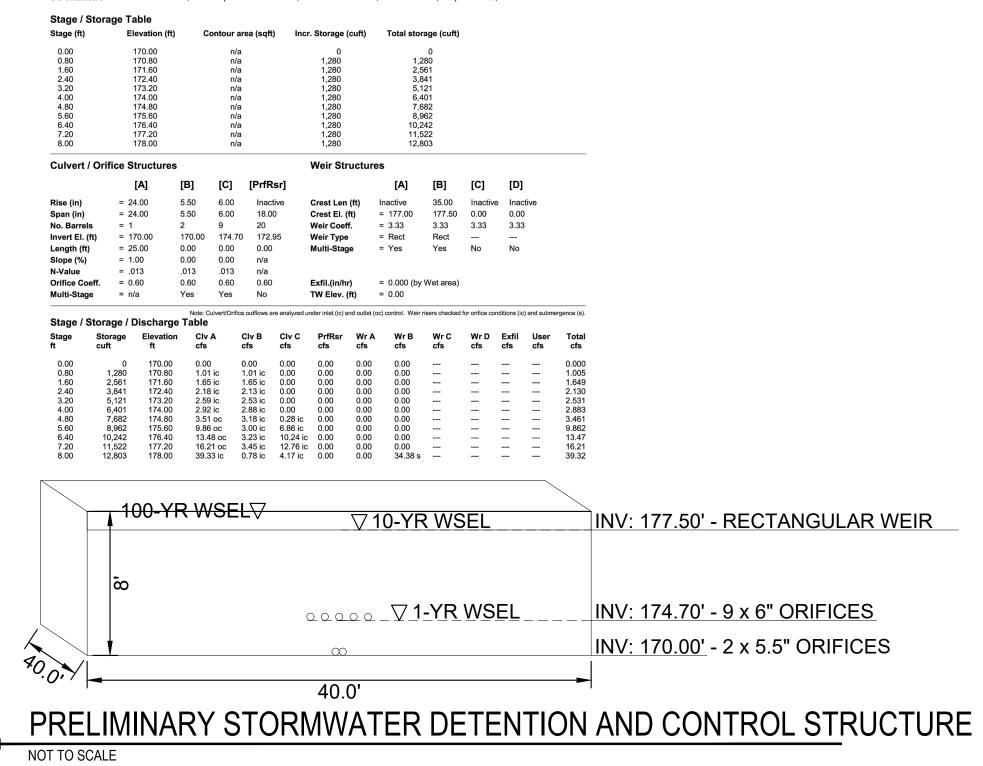
Storm frequency = 10 yrs Time interval = 1 min Inflow hyd. No. = 13 - POST-1 (DETAINED) Max. Elevation = SWM #8 Reservoir name





Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020 Pond No. 1 - SWM #8

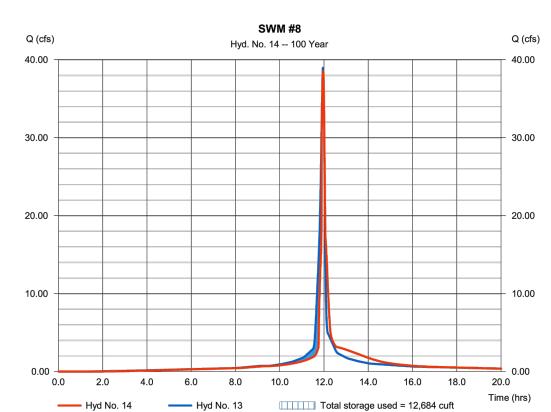
Pond Data UG Chambers -Invert elev. = 170.00 ft, Rise x Span = 8.00 x 40.00 ft, Barrel Len = 40.00 ft, No. Barrels = 1, Slope = 0.00%, Headers = No

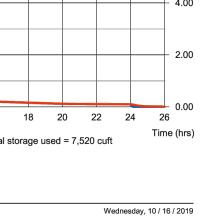


INV: 174.70' - 9 x 6" ORIFICES INV: 170.00' - 2 x 5.5" ORIFICES

INV: 177.50' - RECTANGULAR WEIR

Image: state stat	FLOOD PROTECTION CALCULATIONS
CALL 48 HOURS BEFORE YOU DIG IT'S THE LAW! DIAL 811 Know what's below. Call before you dig.	AGE
APPROVED SPECIAL USE PERMIT NO DEPARTMENT OF PLANNING & ZONING DIRECTOR DATE DEPARTMENT OF TRANSPORTATION & ENVIRONMENTAL SERVICES SITE PLAN NO DIRECTOR DATE CHAIRMAN, PLANNING COMMISSION DATE DATE RECORDED	NEWPORT VILL PREPARED FOR UDR
INSTRUMENT NO. DEED BOOK NO. PAGE NO.	sheet number C-404





= 17.37 cfs

= 12.02 hrs

= 177.51 ft

= 53,328 cuft

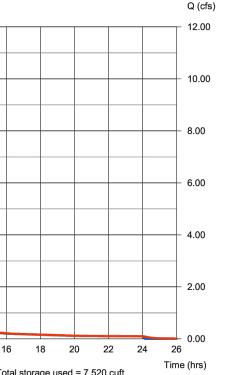
= 12,007 cuft

Peak discharge

Time to peak

Hyd. volume

Max. Storage



Wednesday, 10 / 16 / 2019

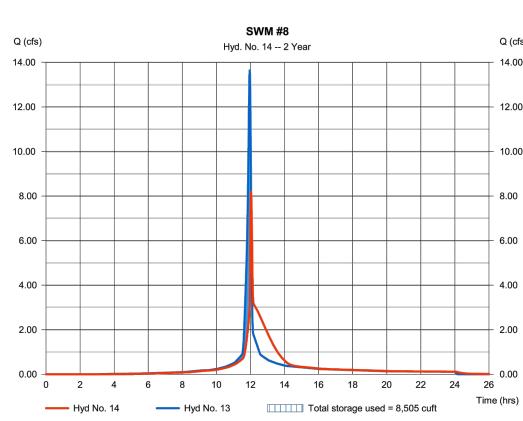
= 3.166 cfs

= 12.08 hrs

= 174.70 ft

= 23,652 cuft

= 7,520 cuft



= 13 - POST-1 (DETAINED) Max. Elevation

Hydrograph Report

Hyd. No. 14

Hydrograph type

Storm frequency

Inflow hyd. No.

Reservoir name

Storage Indication method used.

Hydrograph Report

Hyd. No. 14

Hydrograph type

Storm frequency

Inflow hyd. No.

Reservoir name

Storage Indication method used.

Time interval

SWM #8

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

= Reservoir

= 100 yrs

= SWM #8

= 1 min

Time interval

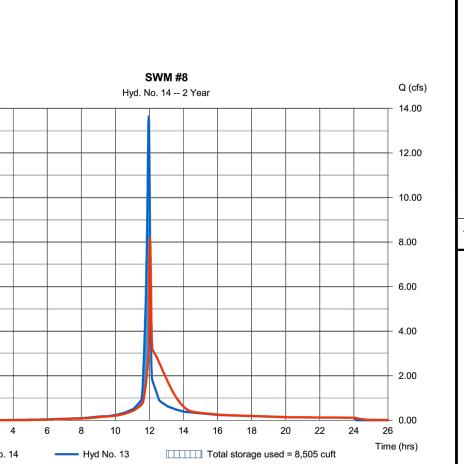
SWM #8

-Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

= Reservoir

= 2 yrs = 1 min

= SWM #8



Peak discharge

Time to peak

Hyd. volume

Max. Storage

Peak discharge

Time to peak

Hyd. volume

Max. Storage

= 8.194 cfs

= 12.03 hrs

= 29,467 cuft

= 8,505 cuft

Wednesday, 10 / 16 / 2019

= 38.54 cfs

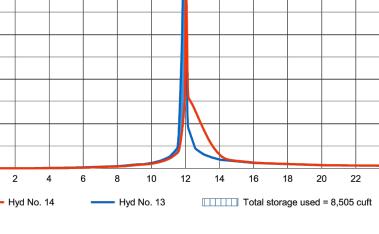
= 11.97 hrs

= 177.93 ft

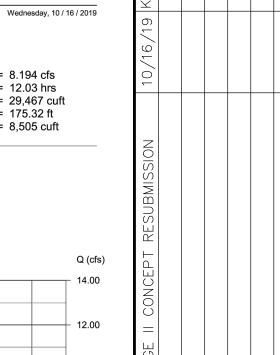
= 12,684 cuft

= 89,821 cuft

= 175.32 ft

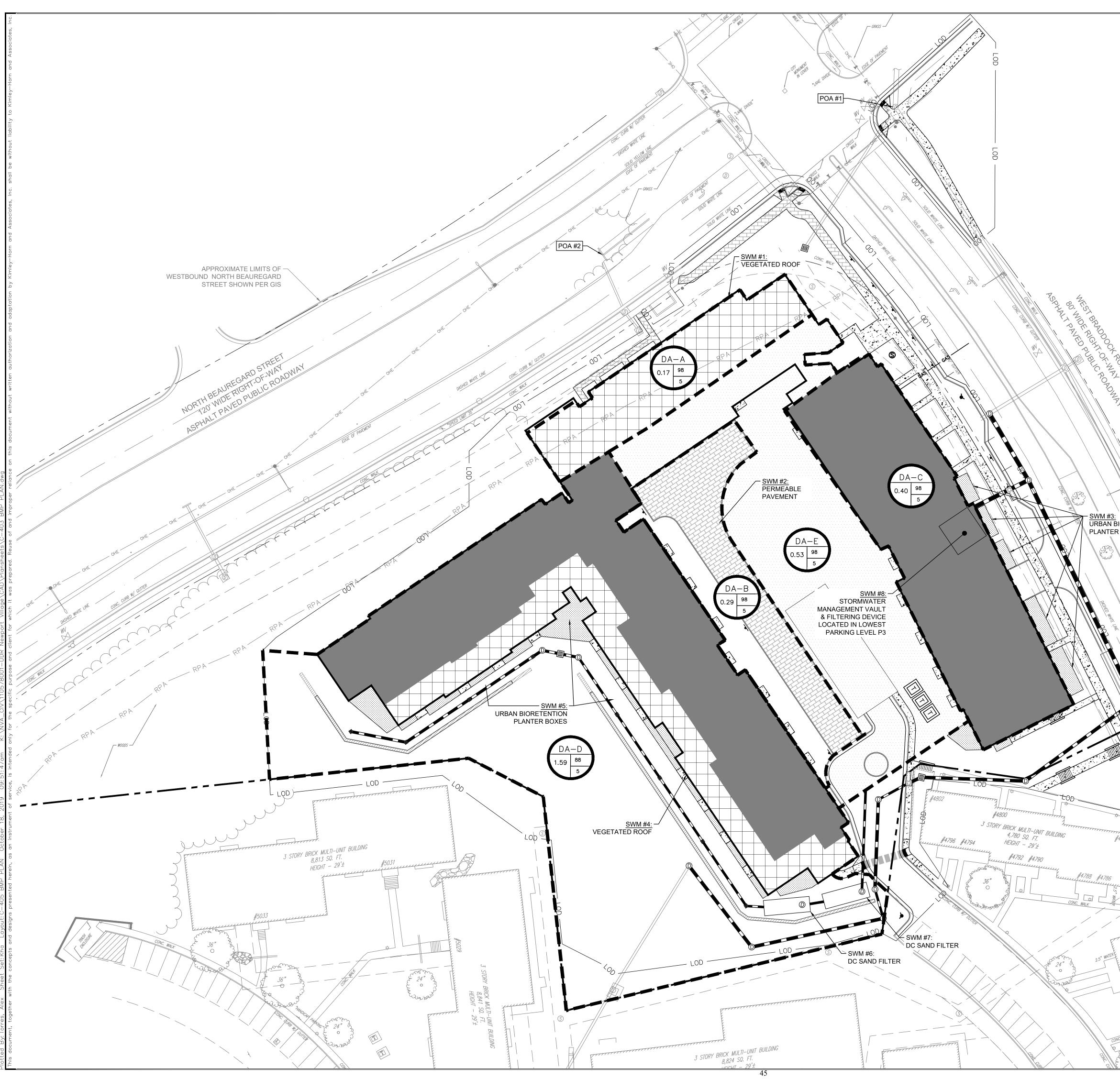


= 13 - POST-1 (DETAINED) Max. Elevation





 $\Omega \square =$



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		/16/19 КНА	DATE BY	
	NORTH IS BASED ON NAD 83 VIRGINIA STATE PLANE NORTH ZONE, US FOOT	Z		
	GRAPHIC SCALE IN FEET 0 15 30 60	SUBMISSION	S Z	
		CEPT RE	REVISIONS)])]
	PERMEABLE PAVEMENT 0.15 ACRES TOTAL	II CONCE		
	ROOF AREA DRAINING TO URBAN BIORETENTION PLANTER BOXES	STAGE		
	IMPERVIOUS AREA DRAINING TO SWM #8 (UNDERGROUND STORMWATER VAULT AND WATER QUALITY FILTERING DEVICE)		6) -
	VEGETATIVE GREEN ROOF 0.35 ACRES TOTAL	UNC	IATES, INC. RESTON, VA 20191	
\ У _Х	URBAN BIORETENTION FILTER BED AREA 0.35 ACRES TOTAL	Hc	() -	
BRADD	MOTES:		RN AND ASSO E, SUITE 400, 03-674-1300 EY-HORN.COM	
UBL OF RO	 STORMWATER BMP DESIGN IS CONCEPTUAL AND SUBJECT TO CHANGE WITH DSUP AND FINAL SITE PLAN. SEE SHEET C-406 FOR BMP CALCULATIONS. THE STORMWATER BEST MANAGEMENT PRACTICES (BMP) REQUIRED FOR THIS PROJECT SHALL BE CONSTRUCTED AND INSTALLED UNDER THE DIRECT SUPERVISION OF THE DESIGN ENGINEER OR THEIR DESIGNATED REPRESENTATIVE. THE DESIGN ENGINEER SHALL MAKE A WRITTEN CERTIFICATION TO THE CITY THAT THE BMPs ARE CONSTRUCTRED AND INSTALLED AS DESIGNED AND IN ACCORDANCE WITH THE APPROVED SITE PLAN. IN ADDITION, AGGREGATE LAYERS AND COLLECTOR PIPES MAY NOT BE INSTALLED UNLESS THE DESIGN ENGINEER OR THEIR REPRESENTATIVE IS PRESENT. 	Kimley»	© 2019 KIMLEY-HORN 11400 COMMERCE PARK DRIVE, PHONE: 703 WWW.KIMLEY	
	 PRESENT. THE CONTRACTOR SHALL FURNISH THE CITY WITH AN OPERATION AND MAINTENANCE MANUAL FOR ALL BMPs ON THE PROJECT. THE MANUAL SHALL INCLUDE AN EXPLANATION OF THE FUNCTIONS AND OPERATIONS OF EACH BMP AND ANY SUPPORTING UTILITIES, CATALOG CUTS ON ANY MECHANICAL OR ELECTRICAL EQUIPMENT AND A SCHEDULE OF ROUTINE MAINTENANCE FOR THE BMPs AND SUPPORTING EQUIPMENT 			_
	WATER QUALITY NARRATIVE: DRAINAGE AREA A: • 0.17 ACRES OF IMPERVIOUS ROOF AREA SHALL BE TREATED THROUGH THE			
- <u>SWM #3:</u> URBAN BIORETENTION PLANTER BOXES	USE OF A VEGETATED ROOF (SWM #1). <u>DRAINAGE AREA B:</u> • 0.29 ACRES OF IMPERVIOUS COURTYARD AREA SHALL BE TREATED			
12. WATER	THROUGH THE USE OF PERMEABLE PAVEMENT (SWM #2). THE PERMEABLE PAVEMENT IS LOCATED WITHIN THE LIMITS OF THE FIRE LANE IN THE COURTYARD. <u>DRAINAGE AREA C:</u>	PROJECT 0578001 DATE /18/2019		
DASHED	 0.40 ACRES OF IMPERVIOUS ROOF AREA SHALL BE TREATED THROUGH THE USE OF URBAN BIORETENTION PLANTER BOXES (SWM #3). THE ROOF WILL BE DESIGNED TO CAPTURE THE ROOF RUNOFF AND DIRECT THIS FLOW TO URBAN BIORETENTIONS ALONG FRONTAGE THROUGH A SYSTEM OF GUTTER DOWNSPOUTS. 	KHA PR 110578 DAT 10/18/	DESIGNED I DRAWN BY CHECKED E	ŗ
DASHED WHITE LINE HITE LINE	 DRAINAGE AREA D: THIS DRAINAGE AREA CORRESPONDS TO A TREATMENT TRAIN WHICH CONSISTS OF VEGETATED ROOF AREA, URBAN BIORETENTION PLANTER BOXES, AND DC SAND FILTERS. 			
	• 0.18 ACRES OF IMPERVIOUS ROOF AREA SHALL BE TREATED THROUGH THE USE OF A VEGETATED ROOF (SWM #4). THEN THE RUNOFF WILL BE DIRECTED TO URBAN BIORETENTION PLANTER BOXES (SWM #5) AND FINALLY DISCHARGE TO PEAT SAND FILTERS (SWM #6 & SWM #7) FOR FINAL TREATMENT.			
	• 0.53 ACRES OF IMPERVIOUS ROOF AREA SHALL BE TREATED THROUGH THE USE OF URBAN BIORETENTION PLANTER BOXES (SWM #5) AND FINALLY DISCHARGE TO DC SAND FILTERS (SWM #6 & SWM #7) FOR FINAL TREATMENT.			
	 0.84 ACRES OF PERVIOUS AREA AND 0.04 ACRES OF IMPERVIOUS AREA WILL BE COLLECTED VIA YARD INLETS AND DRAIN TO DC SAND FILTERS (SWM #6 & SWM #7). DRAINAGE AREA E: 		-	
CONC. CURRA W. CL	• THIS DRAINAGE CONSISTS OF ENTIRELY IMPERVIOUS AREA FROM A PORTION OF THE ROOF, COURTYARD, DRIVE AISLE, AND CONCRETE SIDEWALK. THIS AREA WILL BE COLLECTED INTO SWM #8 AND TREATED BY A FILTERING DEVICE LOCATED WITHIN THE STORMWATER VAULT.			
	 RPA DISTURBANCE TOTAL AMOUNT OF DISTURBANCE IN THE RPA: 15,810 SF TOTAL AMOUNT OF IMPERVIOUS AREA REMOVED FROM THE RPA: 4,144 SF TOTAL AMOUNT OF IMPERVIOUS AREA PROPOSED IN THE RPA: 7,883 SF TOTAL AMOUNT OF REFORESTED AREA IN THE RPA: 1,465 SF 			
#4798	CALL 48 HOURS BEFORE YOU DIG IT'S THE LAW! DIAL 811 Know what's below.	Ш С	A >	, , , ,
788 #4786	APPROVED	-LA		
	DEPARTMENT OF PLANNING & ZONING		\sim	
#4784	DIRECTOR DATE	ORT	D □	1 1
2.5" WATER # #4778 # #4	DEPARTMENT OF TRANSPORTATION & ENVIRONMENTAL SERVICES SITE PLAN NO	ן	ALEXANDRIA	
4,776 4,776	DIRECTOR DATE	EV	OF ALE	
K MULTI-UNI 752 SQ. FT. FEIGHT - 29 WALK	CHAIRMAN, PLANNING COMMISSION DATE DATE RECORDED	Z	CITY 0	
CONC. WALK CONC. CONC.	INSTRUMENT NO. DEED BOOK NO. PAGE NO.	SHEET N	NUMBER 105	

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	<text></text>			10	0/17/2019					constant values				
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Description Description Provide of the state of														
		Post-Development Proje	ct (Treatm				4 10	ī		Chacky				
	<text></text>			Enter		2				ifications List:		t Stds & Specs		
					crease in impervio	us cover (acres) is:	1.24	Lan	d cover areas ente	red correctly?	~			
			Post	t-Developmer	nt TP Load Reducti	ion for Site (lb/yr):	2.75		lotal disturbed	area enterea?	*			
				B Soils	C Soils	D Soils	Totals							
		protected forest/open space or reforested	0.00	0.00	0.00	1.47	1.47	-						
		for yards or other turf to be						1						
		Impervious Cover (acres)	0.00	0.00	0.00	0.99								
Ambiend main strain the strain of a bin d b		Post-Development Land Cover (acr	res)		1			-						
								*						
		Managed Turf (acres) disturbed, graded					1.06							
	<section-header></section-header>	mpervious Cover (acres)				-								
mining m						256 (11)	4.19							
Signification of signific					Runoff Coefficie			1						
	And A and	Target Rainfall Event (inches)	1.00			0.02	0.03	0.04	0.05					
Tentime to the second seco		Total Nitrogen (TN) EMC (mg/L)	1.86											
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				Adjusted ¹	-									
						Cover (acres)			Cover (acres)					
Autor (a) Los Los <thlos< th=""> Los <thlos< th=""> <</thlos<></thlos<>			1		-	% Forest			% Forest					
		Managed Turf Cover (acres)	1.73	1.06			1.06	-		1.06				
Immediations (amp (amp)) 0.00 0.00 Statistical statis s					-									
Implicit and the set of the set					1	Impervious Cover			Re Dev. Impervious		N		r 1.24	
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image:			5,251	4,539		Development Treatment Volume	8,815		Treatment Volume	4,539		Treatment Volume	4,276	
Pre-Robordopment TP Load (Lb/yr) 3.30 2.85 Post-Robordopment TP Load (Tb/yr) 2.69 Pre-Robordopment TP Load (Lb/yr) 0.37	Image: Note Subjective The last of a 3.0 a 2.0 b in the low downwer The 3.0 b is the low downwer The 3.0 b in the low downwer The 3.0 b is the low down were the 3.0 b is the set of 3.0 b is the set of 3.0 b is the set of 3.0 b is the 3.0 b is the 3.0 b is the set of 3.0 b is the 3.0 b is				-			-						
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Adjusted Land Cover Summary: Redevelopment Load TP Load Reduction required for new impervious cover. TP Load Reduction Required for new impervious cover. udjusted total carceage is consistent with Post-ReDevelopment acreage (minus cover (forset/open part) cover (forset open part) cover (fors			tarea and the	1.21						20%				
Auguste Unit Correction Required for Redeveloped Area (lb/yr) 0.57 Required for New Impervious Area (lb/yr) 2.18 Adjusted total acreage is consistent with Post-ReDevelopment acreage (minus acreage of new impervious cover). 0.57 Required for New Impervious Area (lb/yr) 2.18 Column I shows load reduction requirement for new impervious cover (based on new development load limit, 0.41 lbs/acre/year). Post-Development Requirement for Site Area V	Date of the state o			1.21						20%				
Pre ReDevelopment land cover minus pervious land cover (forest/open space or managed turf) acreage proposed for new impervious cover. Required for Rev loped Area (lb/yr) 0.57 Required for New Impervious Area (lb/yr) 2.18	The book point of the real work processes o	Adjusted Land Cover Summary							TP Load Reduction			P Load Reduction		
Adjusted total acreage is consistent with Post-ReDevelopment acreage (minus acreage of new impervious cover). Column 1 shows load reduction requirement for new impervious cover (based on new development load limit, 0.41 lbs/acre/year). Post-Development Requirement for Site Area TP Load Reduction Required (lb/yr) 2.75 Nitrogen Loads (Informational Purposes Only) Pre-ReDevelopment TN load	https://www.initialization.org/angle/ang	Pre ReDevelopment land cover minus pervious		lopen space or					Redeveloped Area	0.57		Impervious Area	2.18	
Column I shows load reduction requirement for new impervious cover (based on new development load limit, 0.41 lbs/acre/year). Post-Development Requirement for Site Area TP Load Reduction Required (lb/yr) 2.75 Nitrogen Loads (Informational Purposes Only) Pre-ReDevelopment TN Load Final Post-Development TN Load	Description Total Total Reduction Regular (lb/y) 2.5		-ReDevelopment ac	creage (minus					(lb/yr)			(lb/yr)		
Post-Development Requirement for Site Area TP Load Reduction Required (lb/yr) 2.75 Nitrogen Loads (Informational Purposes Only) Fre-Re Development TN Load Final Post-Development TN Load	Include Requirement for Site Area The Laad Reduction Required (lb/yr) 2.05 Ditrogen Laads (Informational Purposes Only Colory of a 20.00 Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2" Colspan="2" Colspan="2" Colspan="2">Colspan="2" Colspan="2" Colspan="2" <td co<="" td=""><td></td><td>or new impervious (</td><td>cover (based on</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td>	<td></td> <td>or new impervious (</td> <td>cover (based on</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>		or new impervious (cover (based on									
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Pre-Be Development TN Load	Image: New Laboration of New Laboration 33.62 Image: New Laboration of New Laboration 33.62				TP Load F	Reduction Required	l (lb/yr)	2.75						
Pre-Be Development TN Load	The de blandsprenet TN Load (10/yr) 23.60 Final Post Downlopment TN Isod (10/yr) 33.62													
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ainage Area A Land Cover (acres)	A Soils	B Soils	C Soils	D Soils	Totals	Land Cover Rv]		CLEAR BMP	AREAS			x						
Forest/Open Space (acres) Managed Turf (acres) Impervious Cover (acres)				0.17	0.00 0.00 0.17	0.00 0.00 0.95	-	Total P	hosphorus Ava	ilable for Remo	oval in D.A. A (It	b/yr) 0.37]						
ormwater Best Managem	ent Practi	ces (RR = F	unoff Red	Tot duction)	al 0.17		-				olume in D.A. A	(ft ³) 586	-Select from dropdo	wn lists					
Practice	Runoff Reduction Credit (%)	Managed Turf Credit Area (acres)	Impervious Cover Credi Area (acres	it Upstream	Reduction	Remaining Runoff Volume (ft ³)	Total BMP Treatment Volume (ft ³)	Removal	Phosphorus Load from Upstream Practices (lb)	Untreated Phosphorus Load to Practi (Ib)		By Phosphorus	Downstream Practic Employed	ce to be		Removal fron	Upstream Nit		Nitrogen Ren moved By Nitro actice (Ibs)
egetated Roof (RR) 1.a. Vegetated Roof #1 (Spec #5)	45		0.17		264	322	586	0	Tactices (ib)	0.37	0.17	0.20			1.	Vegetated Roof	(RR)	2.63	1.18
1.b. Vegetated Roof #2 (Spec #5)	60				0	0	0	0		0.00	0.00	0.00				0		0.00	0.00
ainage Area B																			
inage Area A Land Cover (acres)	A Soils	B Soils	C Soils	D Soils	Totals La	nd Cover Rv		CLEAR	BMP AREAS										
Forest/Open Space (acres) Managed Turf (acres)					0.00	0.00													
Impervious Cover (acres)				0.29 Total	0.29	0.95		osphorus Available t Development Tre			0.63								
ormwater Best Managem				Volume from	Runoff R	Remaining Tot	al BMP Phosp	horus Phosphorus	Untreated	Phosphorus		Select from dropdowr	lists	Nitrogen	Niturgen L	oad Untreate	Nitrogen	Remaining	
Practice	Runoff Reduction Credit (%)		Cover Credit		Reduction	Runoff Tre	atment ime (ft ³) Rem Efficience (%	ency Upstream		Removed By Practice (lb)		Downstream Practice Employed		Removal Efficiency (%)	Nitrogen Lo from Upstro Practices (I	am Nitrogen Lo	ad Removed	Nitrogen	
ermeable Pavement (RR) 3.a. Pemeable Pavement #1 (Spec #7)	45		0.29	0	450	550 1	000 25	0.00	0.63	0.37	0.26		3.1	Permeable	Pavement (RI 0.00	4.49	2.64	1.85	
3.b. Permeable Pavement #2 (Spec #7)	75				0	0	0 25		0.00	0.00	0.00			25		0.00	0.00	0.00	
ainage Area C																			
hage Area A Land Cover (acres)	A Soils	B Soils	C Soils	D Soils		nd Cover Rv		CLEAR	BMP AREAS										
Forest/Open Space (acres) Managed Turf (acres) Impervious Cover (acres)				0.40	0.00 0.00 0.40	0.00 0.00 0.95		osnhorus Auril 1	for Parent		0.87								
				Total	0.40	0.50		osphorus Available t Development Tre											
ormwater Best Managem	Runoff	Managed	Impervious	Volume from		-	al BMP Rem	horus Phosphorus		Phosphorus	Remaining	Select from dropdowr		Nitrogen Removal	Nitrogen L		Removed	Remaining	
Practice	Reduction Credit (%)		Cover Credit Area (acres)			Runoff Tre plume (ft ³) Volu	atment	ency Upstream		Practice (lb)	Phosphorus	Employed		Efficiency (%)	from Upstre Practices (I	eam Nitrogen Lo bs) to Practice (Dad By Dractico	P Nitrogen Load (Ibs)	
Doftop Disconnection (RR) 2.a. Simple Disconnection to A/B Soils (Spec #1)	50			0	0	0	0 0	0.00	0.00	0.00	0.00		2	. Rooftop D 0	isconnection 0.00	(RR) 0.00	0.00	0.00	
.b. Simple Disconnection to C/D Soils (Spec #1) .c. To Soil Amended Filter Path as per	25 50			0	0	0	0 0		0.00	0.00	0.00			0	0.00	0.00	0.00	0.00	
cifications (existing C/D soils) (Spec #4) 2.d. To Dry Well or French Drain #1, Micro-Infilration #1 (Spec #8)	50			0	0	0	0 25		0.00	0.00	0.00			15	0.00	0.00	0.00	0.00	
2.e. To Dry Well or French Drain #2, Micro-Infiltration #2 (Spec #8) 2.f. To Rain Garden #1,	90			0	0	0	0 25		0.00	0.00	0.00			15	0.00	0.00	0.00	0.00	
Micro-Bioretention #1 (Spec #9) 2.g. To Rain Garden #2,	40 80			0	0	0	0 25		0.00	0.00	0.00			40 60	0.00	0.00	0.00	0.00	
Micro-Bioretention #2 (Spec #9) .h. To Rainwater Harvesting (Spec #6)	0			0	0	0	0 0		0.00	0.00	0.00			0	0.00	0.00	0.00	0.00	
2.i. To Stormwater Planter, ban Bioretention (Spec #9, Appendix A)	40		0.40	0	552	828	1,379 25	0.00	0.87	0.48	0.39			40	0.00	6.19	3.96	2.23	
ainage Area D inage Area A Land Cover (acres)								CLEAR BN	1P AREAS										
Forest/Open Space (acres)	A Soils	B Soils	C Soils		1	d Cover Rv 0.00					×								
Managed Turf (acres) Impervious Cover (acres)						0.25 0.95	otal Phos	phorus Available fo	r Removal in D	.A. D (lb/yr)	2.10								
ormwater Best Managem	ent Practic	es (RR = R	unoff Redu		1.59		Post D	evelopment Treatr	nent Volume ir	D.A. D (ft ³)	3,349	ect from dropdown lis	te						
Practice	Runoff Reduction		mpervious Ve			maining Total Runoff Treat	BMP ment	al Load from	Phosphorus	Phosphorus F emoved By P	temaining	vnstream Practice to b	e Re	emoval	litrogen Load om Upstream	Untreated Nitrogen Load	Removed	temaining Nitrogen	
egetated Roof (RR)	and the second		Area (acres) P			ume (ft³) Volum	Efficien	cy Upstream Practices (Ib)	Load to		Load (Ib)	Employed			Practices (lbs)	to Practice (Ibs)		Load (Ibs)	
1.a. Vegetated Roof #1 (Spec #5) 1.b. Vegetated Roof #2 (Spec #5)	45 60		0.18		279 0	341 6	21 0		0.39	0.18	0.21 2.i. T	Fo Stormwater Planter (Urb Bioretention)		0		2.79	0.00	1.53 0.00	
ooftop Disconnection (RR)					0				0.00	0.00	0.00		2. R		onnection (RR	- 112-1-00,4-	0.00	0.00	
La. Simple Disconnection to A/B Soils (Spec #1) Lb. Simple Disconnection to C/D Soils (Spec #1)	50 25			0	0	0		0.00	0.00	0.00	0.00			0	0.00	0.00	0.00	0.00	
2.c. To Soil Amended Filter Path as per cifications (existing C/D soils) (Spec #4) 2.d. To Dry Well or French Drain #1,	50			0	0	0) 0	0.00	0.00	0.00	0.00			0	0.00	0.00	0.00	0.00	
Micro-Infilration #1 (Spec #8) 2.e. To Dry Well or French Drain #2,	50 90			0	0	0	25	0.00	0.00	0.00	0.00			15	0.00	0.00	0.00	0.00	
Micro-Infiltration #2 (Spec #8) 2.f. To Rain Garden #1, Micro-Bioretention #1 (Spec #9)	40			0	0		25	0.00	0.00	0.00	0.00			40	0.00	0.00	0.00	0.00	
2.g. To Rain Garden #2, Micro-Bioretention #2 (Spec #9)	80			0	0	0	50	0.00	0.00	0.00	0.00			60	0.00	0.00	0.00	0.00	
.h. To Rainwater Harvesting (Spec #6) 2.i. To Stormwater Planter,	0 40		0.53	0	0 868	0 0		0.00	0.00	0.00	0.00	11.a. Filtering Practice #1		0 40	0.00	0.00	0.00 6.23	0.00	
ban Bioretention (Spec #9, Appendix A) Filtering Practices (no RR)	40		0.55	541	808	1,301 2,1	.09 23	0.21	1.15	0.75	0.01	II.a. Filtering Plactice #1	11.		actices (no RR		0.25	5.51	
11.a.Filtering Practice #1 (Spec #12)	0	0.84	0.04	1,301		2,202 2,2		0.61	0.56	0.71	0.47			30	3.51	4.04	2.26	5.28	
11.b. Filtering Practice #2 (Spec #12)	0			0	0	0 () 65	0.00	0.00	0.00	0.00			45	0.00	0.00	0.00	0.00	
ainage Area E																			
inage Area A Land Cover (acres)	A Soils	B Soils	C Soils	5 D Soils	Totals	Land Cover F			CLEAR BMP	AREAS									
Forest/Open Space (acres)	A JOILS	0.0015		5 0 5013	0.00	0.00													
Managed Turf (acres) Impervious Cover (acres)				0.53 To	0.53	0.00		Fotal Phosphorus				1.15 1,828							
ormwater Best Manage	ment Pra	ctices (RR	= Runoff					Post Develop	ment freatme	volume in	J.A. E (IT)		m dropdown lists				T		
Practice	Runoff Reductio	n Turf Cred	lit Cover Cre		n Reductio	n Runoff	Treatment	2.2	oad from Ph	Load to Re	nosphorus Re moved By Ph	osphorus	am Practice to be imployed		Nitroger Remova Efficienc	from Upstre	am Nitrogen l	Load By Practic	Nitrogen
Manufactured Treatment Device	Credit (% s (no RR)	Area (acre	area (acr	res) Practice (f	t ³) (ft ³)	volume (ft	³) Volume (ft ³)		actices (lb) Pr	Pr	actice (lb)	.oad (lb)			(%)	Practices (I	to Practice	(lbs)	Load (Ibs)
	0		0.53	0	0	1,828	1,828	15	0.00	1.15	0.17	0.97			0	0.00	8.21	0.00	8.21
14.a. Manufactured Treatment Device- Hydrodynamic				0	0	0	0	20	0.00	0.00	0.00	0.00			0	0.00	0.00	0.00	0.00
				0	0	0	0	20	0.00	0.00	0.00	0.00			0	0.00	0.00	0.00	0.00
Hydrodynamic Nanufactured Treatment Device-Filteri				0	0						0.00	0.00			0	0.00	0.00		0.00
Hydrodynamic Manufactured Treatment Device-Filteri				0	ite Res			20 Ility Com	pliance		0.00 D.A.		снеск		0	0.00	0.0		CALL

Area Checks	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	AREA CHECK
FOREST/OPEN SPACE (ac)	0.00	0.00	0.00	0.00	0.00	ОК.
IMPERVIOUS COVER (ac)	0.17	0.29	0.40	0.75	0.53	OK.
IMPERVIOUS COVER TREATED (ac)	0.17	0.29	0.40	0.75	0.53	ОК.
MANAGED TURF AREA (ac)	0.00	0.00	0.00	0.84	0.00	ОК.
MANAGED TURF AREA TREATED (ac)	0.00	0.00	0.00	0.84	0.00	OK.
AREA CHECK	OK.	OK.	OK.	OK.	OK.	
Site Treatment Volume (ft ³)	8,815					-

Runoff Reduction Volume and TP By Drainage Area						
	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	TOTAL
RUNOFF REDUCTION VOLUME ACHIEVED (ft ³)	264	450	552	1,147	0	2,413
TP LOAD AVAILABLE FOR REMOVAL (Ib/yr)	0.37	0.63	0.87	2.10	1.15	5.12
TP LOAD REDUCTION ACHIEVED (lb/yr)	0.17	0.37	0.48	1.63	0.17	2.81
TP LOAD REMAINING (Ib/yr)	0.20	0.26	0.39	0.47	0.98	2.30
NITROGEN LOAD REDUCTION ACHIEVED (Ib/yr)	1.18	2.64	3.96	9.75	0.00	17.54

Total Phosphorus		
FINAL POST-DEVELOPMENT TP LOAD (lb/yr)	5.54	
TP LOAD REDUCTION REQUIRED (lb/yr)		
TP LOAD REDUCTION ACHIEVED (lb/yr)	2.81	
TP LOAD REMAINING (lb/yr):	2.73	
REMAINING TP LOAD REDUCTION REQUIRED (Ib/yr):	0.00	**
** TARGET TP REDUCTIO	N EXCEEDED BY	0.06 LB/YEAR **

REMAINING IT LOAD REDUCTION REQUIRED (ID/ YT).	0.00
** TARGET TP REDUCTION	EXCEEDED
Total Nitrogen (For Information Purposes)	
	20.02

POST-DEVELOPMENT LOAD (lb/yr) 39.62 NITROGEN LOAD REDUCTION ACHIEVED (lb/yr) 17.54 REMAINING POST-DEVELOPMENT NITROGEN LOAD (lb/yr) 22.09

Project Description

Development or Re	edevelopment		
Drainage Area	Impervious	Pervious	Total
Site Area	2.23 AC	1.95 AC	4.18 AC
Dn-Site Treated	2.14 AC	0.84 AC	2.98 AC
Off-Site Treated	0.00 AC	0.00 AC	0.00 AC
otal Treated	2.14 AC	0.84 AC	2.98 AC
Any On-Site Disconnected by a Vegetated Buffer (25 ft)	0.00 AC		
otal On-Site Treated or Disconnected			2.98 AC

<u>Vater Treatment on site</u>

Area treated by BMP (acres)	Impervious area treated by BMP (acres)	BMP efficiency (%)	VRRM Spreadsheet
0.17 AC	0.17 AC	5.80% (0.17 TP)	D.A.A.
0.29 AC	0.29 AC	12.62% (0.37 TP)	D.A.B.
0.40 AC	0.40 AC	16.38% (0.48 TP)	D.A.C.
1.59 AC	0.75 AC	58.01% (1.63 TP)	D.A.D.
0.53 AC	0.53 AC	5.80% (0.17 TP)	D.A.E.
	BMP (acres) 0.17 AC 0.29 AC 0.40 AC 1.59 AC	BMP (acres) treated by BMP (acres) 0.17 AC 0.17 AC 0.29 AC 0.29 AC 0.40 AC 0.40 AC 1.59 AC 0.75 AC	BMP (acres)treated by BMP (acres)0.17 AC0.17 AC0.29 AC0.29 AC0.40 AC0.40 AC1.59 AC0.75 AC58.01%(1.63 TP)

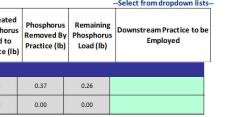
<u>liscellaneous</u>

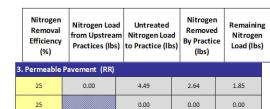
Total WQV treated: Detention on site: yes no yes no

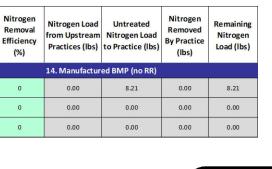
Project is within which watershed? Four Mile Run (West) Watershed

Project discharges to which body of water? Potomac River

46

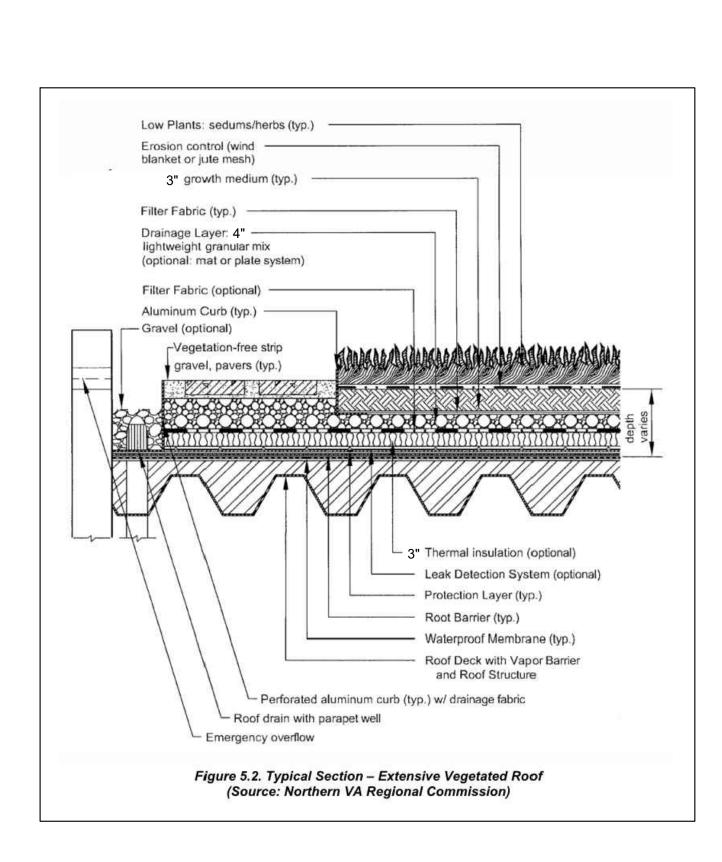




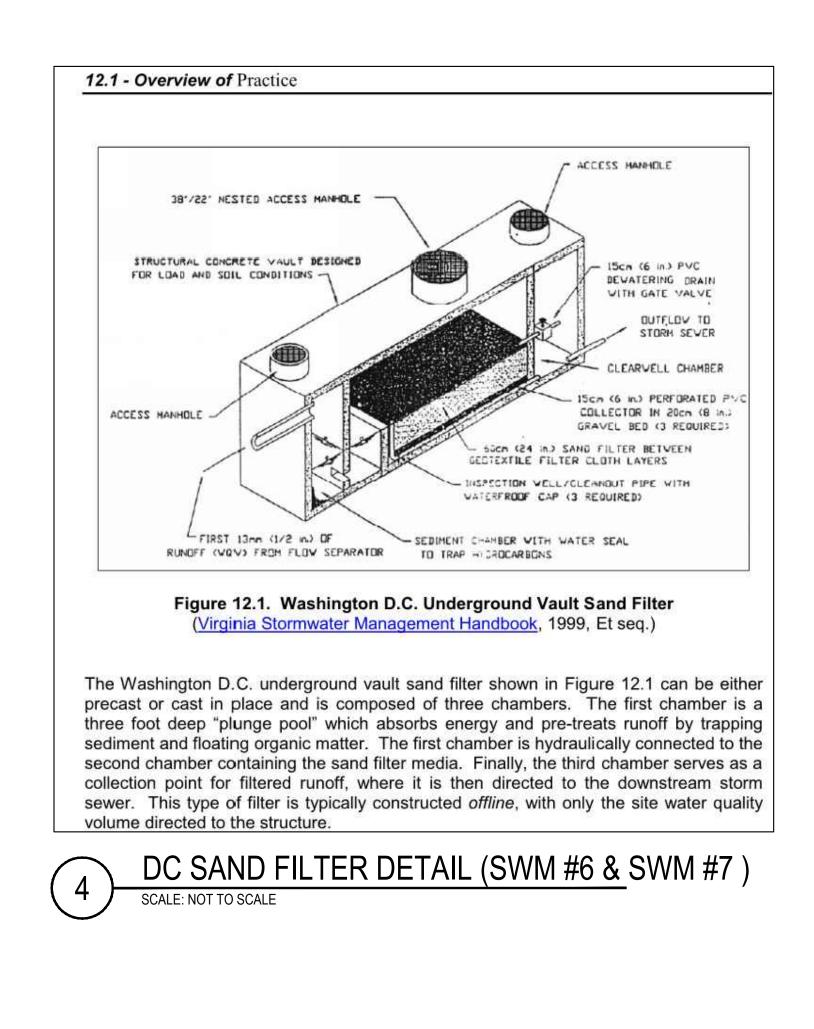


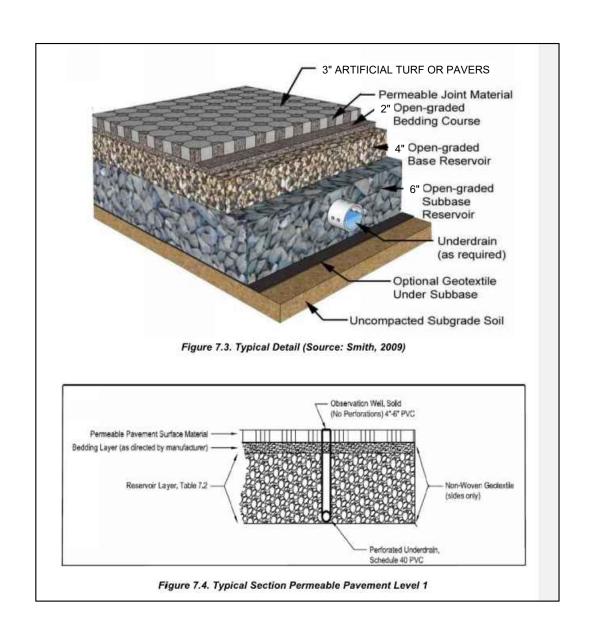


		-Y-HORN AND ASSOCIATES. INC.	11400 COMMERCE PARK DRIVE, SUITE 400, RESTON, VA 20191	PHONE: /03-6/4-1300 WWW.KIMLEY-HORN.COM	No. REVISIONS DATE BY
	DATE 11/07/2019	SCALE AS SHOWN	DESIGNED BY AT	DRAWN BY CEC WWW	CHECKED BY RM
		BMP CALCULATIONS			
u dig.	NEWPORT VILLAGE	PREPARED FOR	UDR		TY OF ALEXANDRIA VA



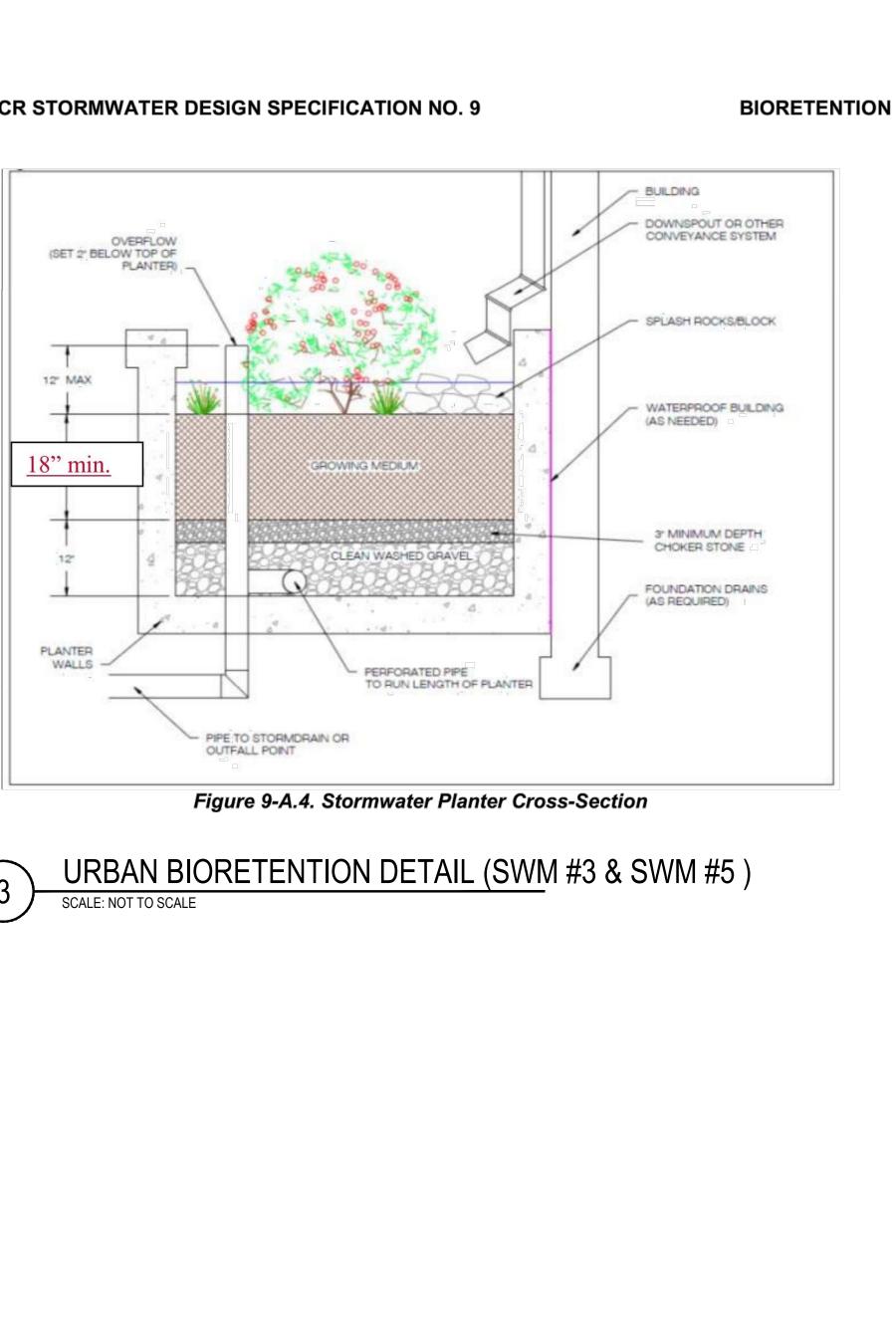
TYPE I GREEN ROOF DETAIL (SWM #1 & SWM #4) SCALE: NOT TO SCALE

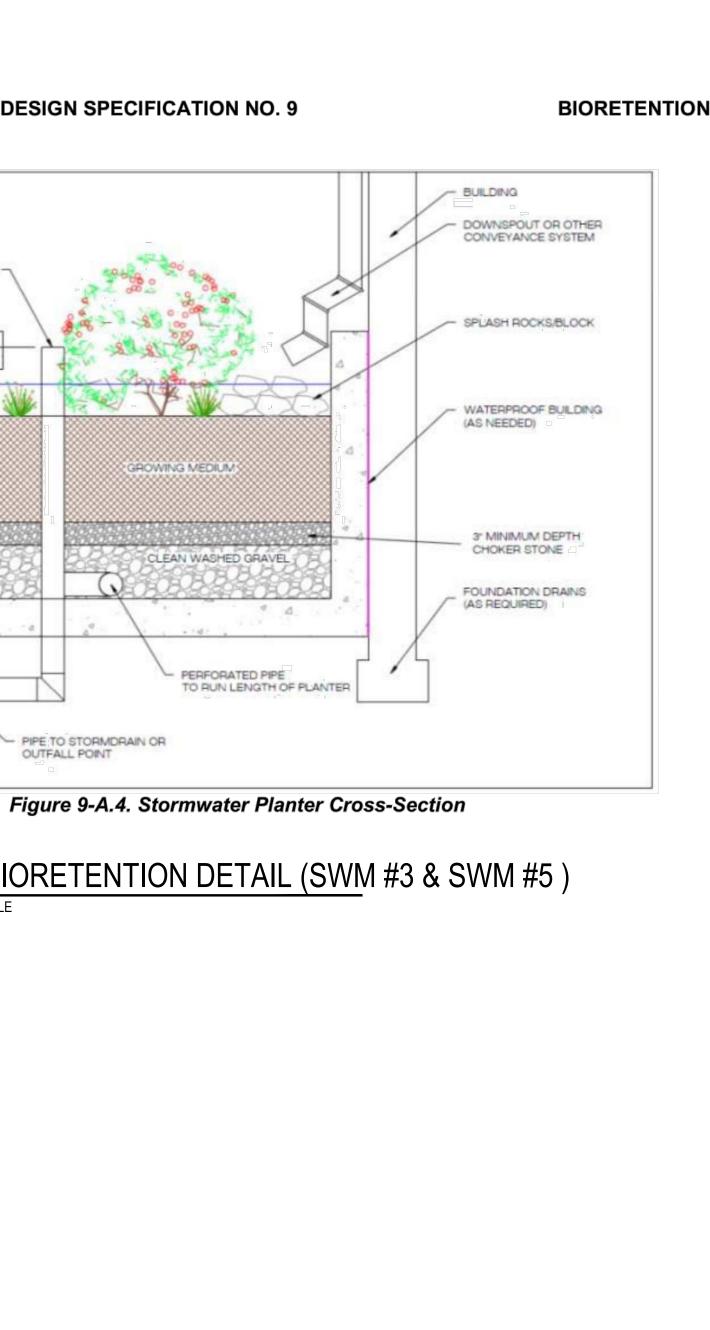


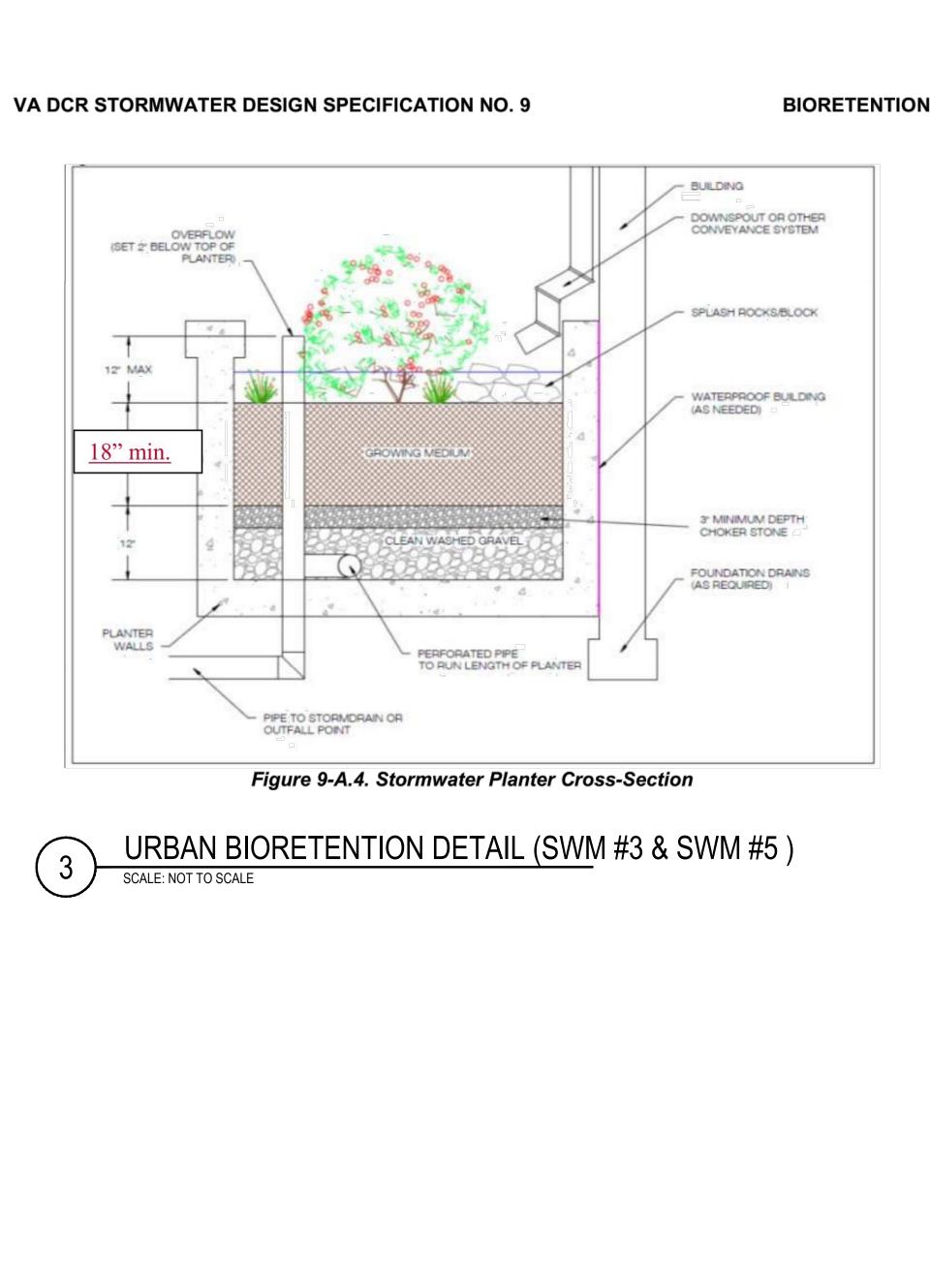


TYPE I PERMEABLE PAVEMENT DETAIL (SWM #2) SCALE: NOT TO SCALE

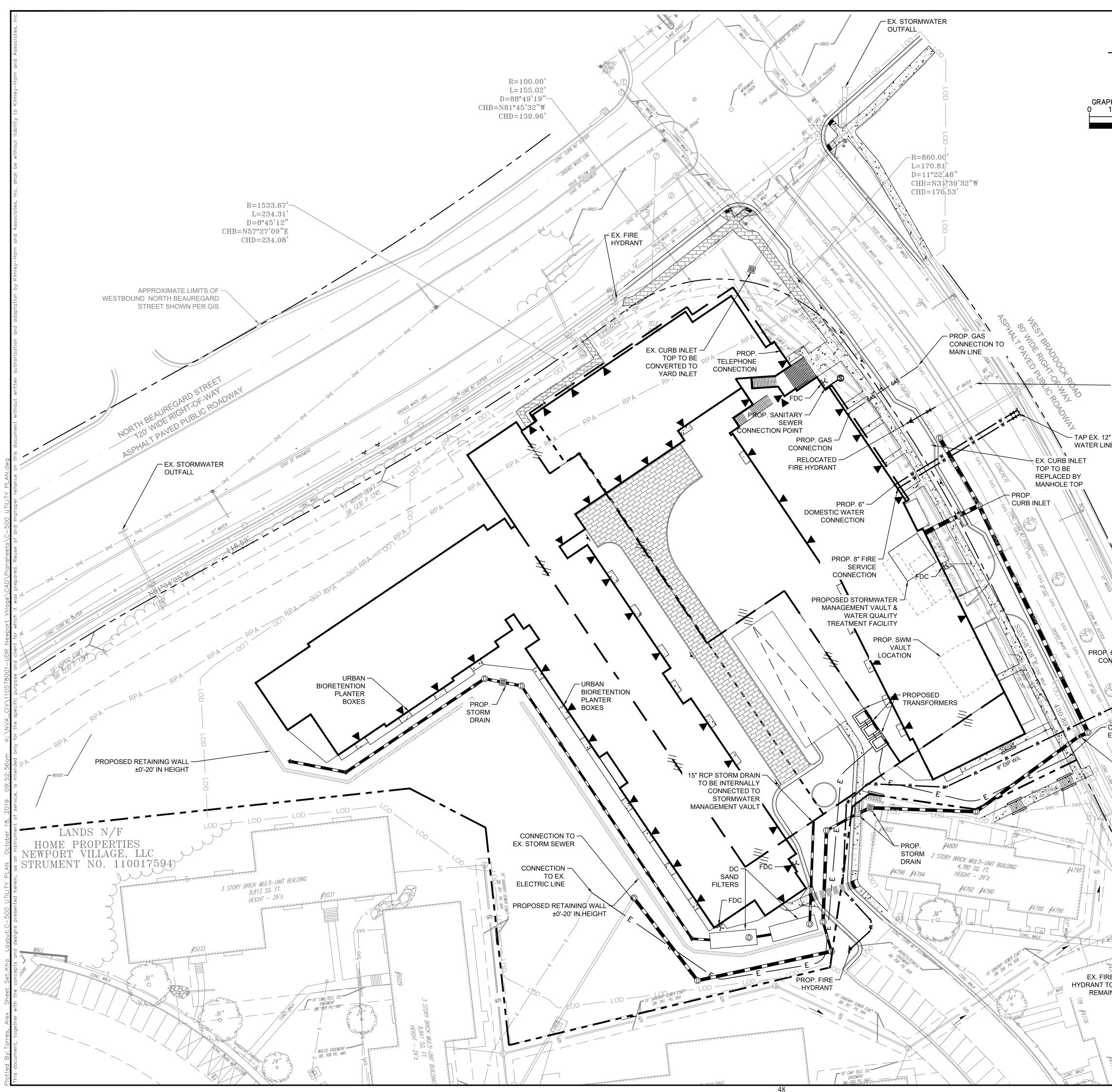
2







	KHA BY
BIORETENTION	10/16/19 DATE
BLOCK	1 STAGE IL CONCEPT RESUBMISSION No. REVISIONS
	Solo KIMLEY-HORN AND ASSOCIATES, INC. © 2019 KIMLEY-HORN AND ASSOCIATES, INC. 11400 COMMERCE PARK DRIVE, SUITE 400, RESTON, VA 20191 PHONE: 703-674-1300 WWW.KIMLEY-HORN.COM
	KHA PROJECT 110578001 DATE 10/18/2019 SCALE AS SHOWN SCALE AS SHOWN DESIGNED BY AT DESIGNED BY AT DRAWN BY CEC CHECKED BY RM
	BMP DETAILS
CALL 48 HOURS BEFORE YOU DIG IT'S THE LAW! DIAL 811	NEWPORT VILLAGE PREPARED FOR UDR
DATE RECORDED	SHEET NUMBER



—	UTILITY	LEGEND	BY KHA
		EXISTING PROPERTY LINE	/16/19 DATE
NORTH NORTH IS BASED ON NAD 83 VIRGINIA STATE PLANE NORTH		PROPOSED SUBDIVISION LINE	
ZONE, US FOOT		PROPOSED ROW DEDICATION	Z
PHIC SCALE IN FEET 15 30 60	LOD	LIMITS OF DISTURBANCE	SUBMISSION
	w w	PROPOSED WATER	PT RESUE
	w w w	EXISTING WATER	ONCEPT
	>	PROPOSED WATER STRUCTURE (VALVE, FH, FDC)	
		EXISTING WATER STRUCTURE (VALVE, FH)	TAGE
	s s	PROPOSED SANITARY SEWER	
	S S	EXISTING SANITARY SEWER	5
		EXISTING SANITARY SEWER	C. VA 20191
		STRUCTURE	ATES, INC. ESTON, V
		PROPOSED STORM SEWER	ASSOCIATES, IN 400, RESTON, -1300 N.COM
		EXISTING STORM SEWER	AND SUITE 5-674-
		STRUCTURE (MH, INLET)	MLEY-HORN PARK DRIVE, PHONE: 703 WWW.KIMLEY
	0 0	EXISTING STORM SEWER STRUCTURE (MH, INLET)	
– EX. 8" WATER LINE TO REMAIN	тт	PROPOSED TELEPHONE LINE	(100 COMMERCE
	ттттт	EXISTING TELEPHONE LINE	11400 C
<u>2"</u> NE	E E	- PROPOSED ELECTRIC	
	ЕЕЕЕ	EXISTING ELECTRIC	
	GAS GAS	PROPOSED GAS LINE	50% SD
	GAS GAS GAS GAS	EXISTING GAS LINE	SET
2. THE AN IS GRE OUTFA SEWER SYSTE 3. WATER 4. THE AF IMMED FOUND ARTIFA POTTE MUST O COMNECTION TO EX ELECTRIC LINE 5. THE AF CONDUCTOR ARCHA	ATER THAN THE 10,000 GPD THAT R LL PER MEMO TO INDUSTRY 06-14. F ADEQUATE OUTFALL ANALYSIS, TH M HAS CAPACITY FOR THE PROPOS METER SIZING TO BE ADDRESSED PLICANT/DEVELOPER SHALL CALL / IATELY (703-746-4399) IF ANY BURIEI DATIONS, WELLS, PRIVIES, CISTERNS ACTSPARTICULARLY PIECES OF WO RYARE DISCOVERED DURING GRO CEASE IN THE AREA OF THE DISCOV S TO THE SITE AND RECORDS THE F	V IS APPROXIMATELY 437,400 GPD. T EQUIRES A STATEMENT ON ADEQUA FOLLOWING A PRELIMINARY SANITA HE RECEIVING SANITARY SEWER ED DEVELOPMENT. WITH NEXT SUBMISSION. ALEXANDRIA ARCHAEOLOGY D STRUCTURAL REMAINS (WALL S, ETC.) OR CONCENTRATIONS OF DRKED QUARTZ, QUARTZITE, OR INE DUND DISTURBING ACTIVITIES. WORL YERY UNTIL A CITY ARCHAEOLOGIST INDS. .LLOW ANY METAL DETECTION TO BI	NIAN
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BRICK MU 4,752	BE IT APPROV SPECIAL USE PE DEPARTMENT OF PE DIRECTOR DEPARTMENT OF TR SERVICES SITE PLAN NO. DIRECTOR DIRECTOR CHAIRMAN, PLANNING C DATE RECORDED	ANNING & ZONING	NEWPORT VILLA PREPARED FOR UDR
11-UNIT BU	INSTRUMENT NO.	DEED BOOK NO. PAGE NO.	sheet number C-500





Kenneth W. Wire kwire@wiregill.com 703-677-3129

September 5, 2019

VIA EMAIL TO karl.moritz@alexandriava.gov

Karl Moritz, Director Planning and Zoning 301 King Street, Suite 2100 Alexandria, Virginia 22314

VIA EMAIL TO yon.lambert@alexandriava.gov

Yon Lambert, Director, Transportation and Environmental Services 301 King Street, Suite 4100 Alexandria, Virginia 22314

RE: Newport Village

Gentlemen:

On behalf of my client, UDR, I am submitting this request for City approval of an encroachment into the Resource Protection Area ("RPA") along the eastern side of Beauregard Street for the above referenced property. The applicant requests this exception to Zoning Ordinance Section 13-107 as permitted by Zoning Ordinance Section 13-119. As shown on the attached exhibits, the total improvements to the property will increase the impervious area within the RPA with offsets proposed by removing City impervious area associated with travel lanes at this intersection and replanting other areas within the RPA. This exception is appropriate because 1) the existing RPA is separated from the stream itself by several lanes of Beauregard Street, 2) the overall water quality will be improved by the proposed mitigation and 3) the proposed building line will create a better interaction between the building façade and the future BRT station.

I. <u>Project Background</u>

UDR is the owner of the Newport Village Apartments and has worked with your staff over the past several months to determine a viable redevelopment strategy for a small portion of the property located at the south western intersection of Beauregard Street and Braddock Road. UDR's initial concept was to demolish the three smaller scale buildings located at this intersection and redevelop their footprint with a larger multi-family building with below grade parking. *See* attached Exhibit A.

As you may know, the grade of the property is very steep as the property rises significantly to the south. During the initial design phase, the UDR team obtained updated construction pricing associated with pushing the new building into the grade which proved to be

September 5, 2019 Page 2 of 3

extremely inefficient and costly. UDR and its team spent the past few months redesigning the new development proposal to move the new building out from the steep slope and save one of the existing buildings by revisiting the proposed project's footprint. *See* attached Exhibit B.

UDR and its team worked with the City to also design a better interaction of the new building with the proposed BRT stop at Beauregard Street and orient the active uses within the building towards the BRT stop. The team also proposes shifting a portion of the building slightly further into the RPA to enhance the pedestrian experience at the intersection and remove the City's existing impervious slip lanes. UDR is supportive of this approach and requests a positive EPC recommendation for the encroachment.

II. <u>Existing RPA Conditions</u>

The proposed redevelopment site is located at the south-west corner of N. Beauregard Street and W. Braddock Road. There is a 100-foot resource protection area (RPA) associated with the stream that runs in the N. Beauregard median. The RPA extends approximately 60 feet into the redevelopment site on the north side. There are existing improvements located within the RPA including the N. Beauregard travel lanes, sidewalks, buildings and other existing infrastructure. Within the redevelopment site, there is one building and its associated concrete walkways located within the RPA. The total on-site impervious area currently within the RPA is approximately 2,400 s.f. (0.06 Ac). *See* attached Exhibit C.

III. <u>Proposed RPA Mitigation</u>

The proposed redevelopment consists of the demolition of 2 existing buildings, associated walkways/sidewalks, parking, drive aisles and utilities, and the construction of a new multifamily building with associated walkways, drive aisles and utilities. Approximately 2,400 s.f. (0.06 Ac) of existing impervious area will be removed from the RPA to make way for the proposed improvements. Approximately 8,700 s.f. of the proposed building and 1,500 s.f. of associated walkways are proposed within the RPA. Therefore, the total on-site impervious area proposed within the RPA is approximately 10,200 s.f. (0.23 Ac).

In order to mitigate the impacts of the proposed improvements within the RPA, the owner is proposing the following (*see* attached Exhibit C):

- 1. **Removal of the existing slip lanes at the intersection of North Beauregard and West Braddock** In order to reduce the impervious surface within the RPA, the two existing slip lanes at the intersection will be removed. The removal of the existing slip lanes will remove approximately 4,356 s.f. (0.1 Ac) of impervious area from the RPA. With the removal of the slip lanes, vehicular contact areas will be removed from the RPA and replaced with pervious cover. This will reduce the amount of high-concentration pollutants generated by surface runoff being conveyed into the RPA.
- 2. **Reforestation** For any additional areas of impacts within the RPA (estimated as 0.07 Ac), reforestation using native species will be provided within the RPA (approximately 3,300 s.f. is proposed).

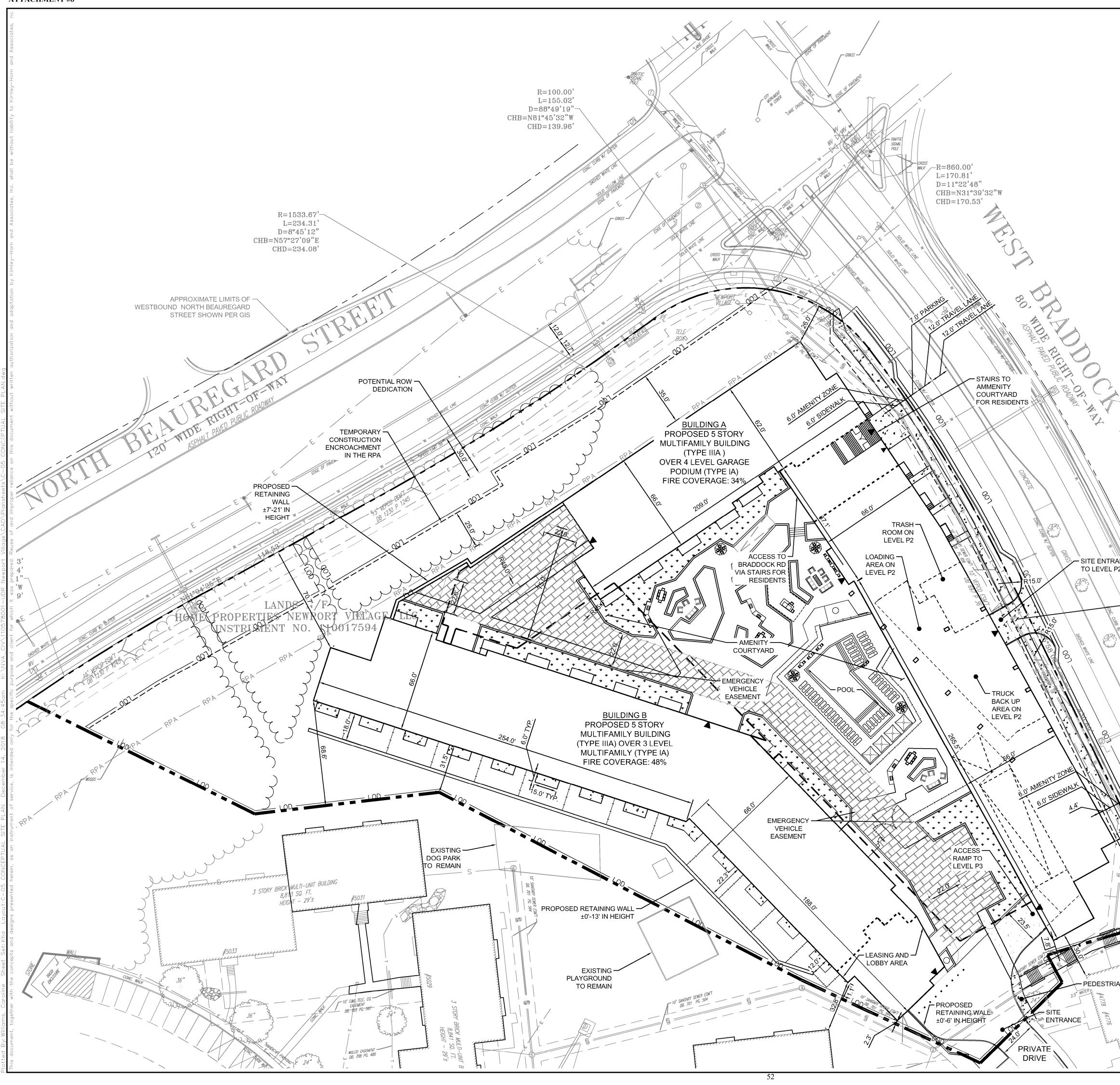
September 5, 2019 Page 3 of 3

> 3. Concentrate Green Roof Areas within the RPA – In an effort to reduce stormwater runoff into the RPA and mimic natural ground cover, green roof areas on the roof of the proposed building will be concentrated in the portion of the building located within the RPA.

For each of these reasons, we respectfully request City approval of an exception to Zoning Ordinance Section 13-107 for the proposed encroachments and improvements within the RPA. If the EPC finds that is cannot recommend support of the proposed encroachment, UDR will proceed with a slightly revised project layout which removes all additional encroachments from the RPA and leaves the existing slip lanes and off-site RPA as is (Exhibit A).

Sincerely,

By: Kenneth W. Wire, Wire Gill LLP



]		B
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